Central Queensland Coal Project Appendix 6 - Groundwater Technical Report

Supplementary Environmental Impact Statement

Central Queensland Coal Project **Appendix A6 – Groundwater Technical Report**

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1 Introduction

This report provides technical details that support Chapter 10 – Groundwater of the Supplementary Environmental Impact Statement (SEIS) for the Central Queensland Coal Project, and includes:

- 1. Section 1- Introduction
- 2. Section 2 – 2018 Hydraulic conductivity test (slug and recovery test) analyses and results Details of aquifer testing methods, analysis methods, and resulting hydraulic conductivity estimates
- 3. Section 3 Numerical Model Details of the numerical groundwater modelling undertaken to simulate the Project area groundwater system, mine water affecting activities and potential direct groundwater effects associated with the Project
- 4. Section4 Styx River instream pools water balance Details the processes that sustains the persistence of water pools during the dry season along Deep creek and Tooloombah creek. Provide a preliminary quantification of the various sources of water to the pools and an assessment of the expected impact from the mining activities on those pools.
- 5. Section 5 Targeted GDE investigations Details the targeted investigations were undertaken in August 2018 to better understand vegetation water use.
- 6. Section 6 Connectivity assessment using isotope analysis Details isotope studies undertaken to better understand the relationship between the surface water and groundwater,

The information and data presented in this technical report provides baseline data of relevance to Chapter 10 of the SEIS, where additional detail concerning the physical setting of the Project that is of importance for hydrogeological characterisation and conceptualisation.

2 Aquifer testing

2.1 Overview

Slug tests and injection recovery tests have been performed on newly installed Styx WMP bores to derive estimates of hydraulic conductivity (K) of the strata directly adjacent the bores. The bores have been screened in the Alluvium and Styx Coal Measures, and at some locations the bores straddle the Alluvium and upper parts of the Styx Coal Measures. Three slug test methods were used; a near-instantaneous injection of potable water was added to a bore to displace the water column (falling head test); a volume of water was removed from the bore instantaneously (using a bailer) (rising head test); or a volume of water was removed from the bore using drilling rig to airlift the bore dry (rising head test). For bores that exhibited a quick recovery during testing that made analysis of the recovery data impracticable, a constant rate recovery test analysis was undertaken. In each method the water level recovery was monitored back to static.

The following provides a description of methods and a summary of results. The bore logs are provided in Attachment 1 and graphical representation of the testing data and results for the tested bores is provided in Attachment 2.

2.2 Analysis methods

2.2.1 Solutions

2.2.1.1 Slug testing using the Hvorslev (1951) and Bouwer-Rice (1976) solutions

Hvorslev (1951) and Bouwer-Rice (1976) developed mathematical solutions for deriving estimates of hydraulic conductivity values for low to moderate permeability aquifers based on water level response to falling and rising head tests (i.e. slug tests).

The method developed by Hvorslev (1951) is applicable for slug tests of partially penetrating wells or piezometers. The method is intended for confined aquifers but can yield reasonable results for unconfined aquifers if the screened interval is sufficiently far from the water table and data are corrected for construction limitations.

The method developed by Bouwer-Rice (1976) is applicable for slug tests of fully or partially penetrating wells or piezometers installed in unconfined aquifers. However, reasonable results can also be obtained for tests conducted on wells or piezometers in confined aquifers.

2.2.1.2 Constant rate recovery testing: Theis (1935)

Theis (1935) developed a mathematical solution for determining aquifer transmissivities from the recovery response at the termination of pumping / injection tests. The solution is intended for analysing results from tests conducted on non-leaky confined aquifers and is suitable for tests where pumping / injection rates are variable, as long as the total volume of abstractions or injection, and the time of testing is known.

2.2.2 Method

All three solutions were implemented by fitting a straight line to water level response data. For slug test analyses, AQTESOLV (Duffield, 2007), an industry standard aquifer test analysis software was implemented. AQTESOLV was used to fit a straight line to a log-log plot of normalized head (i.e. relative to initial displacement) and elapsed time. Straight lines were fitted to early, mid and late time to obtain ranges for hydraulic conductivity estimates for the Bouwer-Rice solutions, and straight lines were fitted to the entire (0.1 to 1 displacement) data set for the Hvorslev solutions.

For constant rate recovery tests, manual methods were used to fit a straight line to mid- and latetime data on a semi-log plot of residual drawdown against t/t' (the ratio of total elapsed time since testing commenced and time since the end of testing).

2.2.3 Analysis summaries and results

Details of slug and constant rate recovery test analyses and the derived estimates of Ks for the bores tested are provided in Attachment 2, and a summary is presented in [Table](#page-11-0) 2-1.

Bore ID	Analysis Method	Test type	Screened Stratigraphy ¹	Solution	Estimated K, m/d ²	b, m ²	Estimated T,m ² /d
WMP02	Recovery test	Rising head	Alluvium	Theis Recovery	5	8.5	43
WMP04	Slug test	Falling head	Alluvium	Bouwer-Rice	0.01	3.9	0.04
				Hvorslev	0.02		0.08
WMP04D	Slug test	Falling head	Alluvium / Styx Coal Measures - overburden	Bouwer-Rice	0.02	23.2	0.5
				Hvorslev	0.03		0.7
WMP05	Slug test	Falling head	Alluvium	Bouwer-Rice	0.03	4.9	0.1
				Hvorslev	0.07		0.3
WMP06	Slug test	Falling head	Alluvium / Styx Coal Measures - underburden	Bouwer-Rice	0.01	1.5	0.02
WMP08	Slug test	Falling head	Alluvium	Bouwer-Rice	0.0005	3.9	0.002
WMP08D	Slug test	Falling head	Styx Coal Measures - underburden	Bouwer-Rice	0.03	26.5	0.8
WMP09	Slug test	Falling head	Alluvium	Bouwer-Rice	0.1	4.2	0.5
				Hvorslev	0.2		0.9
WMP10	Slug test	Falling head	Styx Coal Measures - overburden	Bouwer-Rice	0.004	10.7	0.04
WMP12	Recovery test	Falling head	Alluvium / Styx Coal Measures - overburden	Theis Recovery	$\overline{2}$	8.5	17
WMP13	Slug test	Falling head	Alluvium / Styx Coal Measures - overburden	Bouwer-Rice	0.3	5.4	$\overline{2}$
WMP16	Slug test	Falling head	Styx Coal Measures- overburden	Bouwer-Rice - Early	0.3	18.3	5
				Bouwer-Rice - Late	0.2		$\overline{4}$
		Rising head		Bouwer-Rice - Early	0.2		$\overline{4}$
				Bouwer-Rice - Mid	0.04		0.7
				Bouwer-Rice - Late	0.007		0.1
WMP16D	Slug test	Falling head	Styx Coal Measures - interburden	Bouwer-Rice - Early	0.02	28.6	0.6
				Bouwer-Rice - Mid	0.003		0.1
				Bouwer-Rice - Late	0.001		0.03

Table 2-1 Summary of derived hydraulic property estimates from aquifer tests

3 Numerical groundwater flow model

3.1 Overview

Groundwater modelling is the only practical approach for assessing the potential effects of the Project on future groundwater conditions, dependent systems (surface water and ecological), and Styx Basin Environmental Values (EVs) in response to mine water affecting activities. This section provides a technical account of the numerical groundwater modelling that has been conducted for the Project. The conceptual modelling and the key results from predictive simulations are presented in Chapter 10 – Groundwater of the (SEIS).

This section provides details of the following:

- Model construction and calibration (Section [3.4](#page-21-0) and [3.5\)](#page-30-0) describes how the numerical groundwater flow model is constructed, the hydrogeological processes that are represented within the model, and the ability of the model to simulate past groundwater conditions as an indication of its suitability for predictive simulation.
- **•** Predictive simulations (Section [3.6\)](#page-43-0) describes how the model is built to simulate the potential effects of the Project on groundwater conditions including potential changes to water table elevation, groundwater pressures, and groundwater recharge and discharge processes, both during and after mining.
- Sensitivity and uncertainty analysis (Section [3.7\)](#page-89-0) describes how parameter sensitivity and predictive uncertainty has been conducted, and how the numerical model (and predicted potential effects on groundwater) responds to altered parameters and possible conceptualisations.

3.2 Modelling objectives

The primary objectives of the groundwater modelling are to:

- **•** Predict the potential drawdown of the water table and depressurisation of the local (mine-scale) and regional groundwater system due to mine dewatering, and to provide the basis for
	- a. Assessing potential impacts on existing groundwater users (environmental and economic) and EVs
	- b. Informing groundwater monitoring and management requirements / commitments.
- Predict potential inflows of groundwater (dewatering requirements) to the proposed mine pits for input to the Water Management Plan, which will be implemented throughout the life of the Project.
- **•** Predict recovery of groundwater pressures and potential long-term changes in groundwater conditions as
	- a. the mine pits are progressively backfilled; and
	- b. after mining has ceased.

3.3 Groundwater modelling guidelines

3.3.1 Background

The practice of groundwater modelling in Australia has been influenced by guidelines developed by the Murray-Darling Basin Commission (Middlemis et al. 2001), widely known as the "MDBC guidelines", and more recently by The Australian Groundwater Modelling Guidelines (Modelling Guidelines) released by the National Water Commission (Barnett et al. 2012). The guidelines were designed to reduce the level of uncertainty surrounding modelling by promoting transparency in methodologies and encouraging consistency and best practice. The Modelling Guidelines are similar to, and broadly consistent with, the earlier MDBC guidelines, providing information that is useful for all stakeholders regarding the outcomes of groundwater modelling and the reliance on results that can be achieved - from proponents of projects to regulators to professional groundwater modellers, and to members of the community.

The groundwater modelling detailed here has been undertaken in a manner consistent with the methods and recommendations of the Modelling Guidelines.

3.3.2 Model confidence level classification

The degree of confidence with which a model's predictions can be used is a critical consideration for any groundwater modelling exercise. Several factors are typically considered to determine model confidence level classification, and the Modelling Guidelines (Barnett et al. 2012) define a rigorous system to classify the confidence level of groundwater models based on the following factors:

- Available data;
- Calibration procedures;
- **•** Calibration and prediction consistency; and
- Level of stress (i.e. hydraulic stress in the model).

Models are classified as Class 1, 2 or 3 in order of increasing confidence (and, often, complexity). In general, a model will not fit entirely into one class of confidence level because determining the most appropriate class depends upon multiple factors. [Table](#page-17-0) 3-1 presents a review of Styx model compliance with modelling guidelines criteria (see Table 2-1 on pages 20 to 21 of the guidelines). The Styx groundwater model is consistent with criteria spanning over the 3 confidence levels. The model clearly complies or exceeds all criteria for a Class 1 model. It is mostly consistent with a Class 2 model and the data spatial distribution and availability are partially consistent with a Class 3 model. The confidence classification scheme in the modelling guidelines does not allow for a full compliance with a Class 2 or 3 confidence level for large regional-scale groundwater systems having long response and recovery times, and when the strata targeted for development and depressurisation hasn't been previously stressed at the magnitude expected of the project under consideration.

More generally, the Class 1 groundwater model developed for the Project is considered capable of providing appropriate physically-based predictions of relative responses to hydraulic stresses. On this basis, it is considered fit-for-purpose and an appropriate platform for assessing the potential impacts of the Project on existing and future groundwater conditions. As the mining project develops, monitoring of groundwater system response will further inform the certainty around model predictions and, so, the class of model will evolve to Class 2 and 3.

Table 3-1 Modelling guidelines characteristics and indicators for Styx model confidence level classification (Legend at the bottom of the table)

Partially achieved or not critical for Project objective(s)

Styx model exceeds the guideline criteria

Legend:

Not achievable for the current Styx model

Consistent with the Styx model

Partially achieved or not critical for Project objective(s)

Styx model exceeds the guideline criteria

Legend:

Not achievable for the current Styx model

Consistent with the Styx model

Partially achieved or not critical for Project objective(s)

Styx model exceeds the guideline criteria

Consistent with the Styx model

Partially achieved or not critical for Project objective(s)

Styx model exceeds the guideline criteria

3.4 Model construction

3.4.1 Overview

This section of the report describes how the numerical groundwater flow model has been constructed, particularly in regard to its representation of the conceptual hydrogeological model (presented in Chapter 10 of the SEIS, Section 10.5.6) and how the numerical model is calibrated to simulate existing groundwater conditions as an indication of its suitability for conducting predictive simulations. The geometry of the groundwater model is constructed from the regional-scale geological model developed for the Project, which utilised geological data and interpretations provided by CQC (for the Styx geological basin) and on published geological maps (1:250,000 St Lawrence geological map, sheet SF 55-12, 1970).

3.4.2 Regional-scale geological model

A regional-scale hydrostratigraphic model has been constructed using Leapfrog Hydro (v2.8) to support the construction of the groundwater model. The primary sources of information used for the geological model include the geological information discussed in Section 10.5.5 of the SEIS (including the surface and solid geological maps), lithological logs recorded in the GWDBQ and for recently drilled Project bores (WMP bores), as well as the upper and lower surfaces of coal seams from the Proponent's local-scale geological model. The regional surface geology, along with the geological model extent and Project Mining Lease (ML) is presented on [Figure 3-1.](#page-22-0)

The regional-scale model consists of four main hydrostratigraphic units (HSUs) shown in [Figure](#page-23-1) [3-2:](#page-23-1)

- **HSU1** Cenozoic surface deposits occurring mainly in association with alluvial infill deposits and colluvial slope deposits associated with the surface drainage network.
- HSU2 Styx (geological) Basin consisting of the Cretaceous Styx Coal Measures, which are sub-divided into
	- a. Overburden consisting of the portion of the Styx Coal Measures above the upper-most coal seam delineated in the Proponent's local-scale geological model
	- b. Coal seams and interburden consisting of the portion of the Styx Coal Measures between the upper and lower coal seams as delineated in the Proponent's local-scale geological model
	- c. Underburden consisting of the portion of the Styx Coal Measures below the lowermost coal seam delineated in the Proponent's local-scale geological model.
- HSU3 Outcropping and sub-cropping weathered basement (Back Creek Group, Lizzie Creek Volcanic Group and Connors Volcanic Group)
- HSU4 Basement consisting of Permian and older rocks that either underlie the Styx (geological) Basin, or sub-crop beneath Cenozoic sediments, or outcrop beyond the margin of Styx Basin, including undifferentiated Back Creek Group, Boomer Formation (Back Creek Group), Carmila Beds (Lizzie Creek Volcanic Group) and Connors Volcanic Group.

Each HSU represented in the Leapfrog Hydro model is a 3D volume that can be continuous or discontinuous within the geological model domain. The unit thicknesses and the contact surfaces between units are represented by the software based on interpolation and extrapolation of the input data, which include manually drawn lines and intersections, and specified stratigraphic relationships between the units.

Conceptual geological and hydrostratigraphic cross-section line ML 700022

Styx Local Geological Model

• ML 80187

Figure 3-1 Regional surface geology

DATA SOURCE QLD Open Source Data, 2018; Waratah Coal, 2018 Styx basin modified from Central Queensland Coal and Qld Open Source Data, 2018; St. Lawrence 1:250k geological map, BoMN, 1970; Geofabric v2.1, Bureau of Meteorology, 2012

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10 km

Date:

Scale @ A4 1:300,000 28/11/18

Drawn: A. Aird

----- Geological structure Main road

Waterbody

Figure 3-2 Regional-scale geological model (x5 vertical exaggeration)

3.4.3 Numerical groundwater flow model

3.4.3.1 Modelling platform

Several industry-standard model codes are available for simulation of regional-scale groundwater flow. They roughly fall into two categories - finite element (FE) and finite difference (FD). The MODFLOW FD model code was selected for this modelling project, the code was developed by the United States Geological Survey (USGS; McDonald and Harbaugh 1988). A recent version of MODFLOW called MODLFOW-USG (Unstructured Grid, first released in 2013) comprises older versions of MODFLOW but permits the development of a grid having cells of variable dimensions within the grid, much the same as can be achieved by use of a FE model code.

MODFLOW-USG has been chosen as the most appropriate numerical modelling package for this project. It was developed by the USGS (Panday et al., 2013) and has the following advantages:

- It handles issues associated with re-wetting of dry cells more effectively than previous MODFLOW versions.
- It allows for local refinement of the grid, which offers the possibility to progressively increase the refinement of the grid around the area of interest.

Groundwater Vistas version 6 (ESI 2011) was selected as the graphical user interface for building the model and some of the associated pre‐ and post-processing. Additional scripts were written in Excel VBA, Python and ArcGIS™ to perform customised pre- and post-processing tasks.

3.4.3.2 Model discretisation

Model Grid

The model domain extends about 43 km north to south and 54 km east to west. The model grid (Quadtree mesh, which allows grid cell refinement as a multiple of previous cells.) shown in [Figure](#page-24-0) [3-3](#page-24-0) and [Figure 3-4](#page-25-0) was generated in Groundwater Vistas. The columns are aligned due north and are approximately parallel to the regional orientation of the Styx Basin and the bounding Permian rocks, and the general regional strike of groundwater flow. The total number of active cells is 466,638, which is made up of six layers each comprising 77,773 cells.

Local refinement is introduced within and surrounding the Project area. The 5×5 km area representing ML 80187 has a uniform grid cell size of 40×40 m. The cells around Tooloombah and Deep Creeks that border the lease have a refinement of 20x20 m. The size of grid cells progressively increases to 40 m, 80 m, 160 m, 360 m and 640 m with increasing distance from the Project area.

Figure 3-3 MODFLOW USG Quadtree Grid (model Domain)

Figure 3-4 Close-up of the MODFLOW USG Quadtree grid centred on the refined area

3.4.3.3 Model layering and hydrostratigraphic units

The model comprises six layers, as summarised i[n Table](#page-27-1) 3-2. The top surface of the model is set at topographic elevation and the bottom elevation of the model is set arbitrarily at -500 mAHD, within Permian rocks. The topographic elevation was constructed by merging information from three topographic datasets of varying definition (LIDAR: 1m resolution, DEM: 10m resolution and STRM: 90m resolution). The more precise resolution was used where available, typically within the Project area. The area covered by each dataset is illustrated on [Figure 3-5.](#page-26-0) The HSUs described in Section 10.5 Hydrogeology of the SEIS Chapter 10 align with the model layering.

The expected range in hydraulic properties (horizontal hydraulic conductivity (Kh), vertical hydraulic conductivity (Kv) and specific yield (Sy)) are presented i[n Table](#page-27-1) 3-2 and these parameters were assessed from the aquifer testing undertaken for the Project (see Section 2), regional aquifer testing results and as part of model calibration. The specific storage (Ss) value for all HSUs has been conservatively set at the value of water compressibility $(5 \times 10^{-6} \text{ m}^{-1})$.

3.4.3.4 Temporal discretisation

The model comprises an initial stress period to establish steady state pre-mine (baseline) conditions followed by monthly stress periods covering the period from 01/2017 to 03/2018. The steady state conditions are reached by applying average climatic stress for a 10,000 year period over a single transient stress period - refer to hydrographs in [Figure 3-45](#page-74-0) to [Figure 3-47,](#page-76-0) which demonstrate steady state conditions (baseline) at the start of mining. This solution was favoured above a numerical steady state solution approach due to possible numerical instability and lack of reliability of the evapotranspiration (ET) package in steady state.

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Table 3-2 Model layers and expected range of hydraulic properties

Notes: 1 The fractured and weathered basement zone is only present where basement (and a small portion of Styx Coal Measures) outcrops

2 Kh – horizontal hydraulic conductivity; Kv – vertical hydraulic conductivity; Sy – specific yield (dimensionless), Ss – specific storage

Min = minimum; Max = Maximum

3.4.4 Model boundary conditions

The model boundary conditions (BCs) are specified as described in the conceptual hydrogeological model section of Chapter 10 - Groundwater (Section 10.5.6.8). The BCs are illustrated on [Figure 3-6](#page-28-0) and discussed further below.

3.4.4.1 Model external boundaries and Broad Sound

The western, eastern and southern parts of the model domain follow the ridgeline of the Styx River Basin [\(Figure 3-5\)](#page-26-0), which includes the tributary catchments of Tooloombah and Deep Creeks within which the Project is located. These BCs have been defined as no-flow boundaries because the elevation of the water table is topographically controlled, and the ridge line of the Styx River catchment is also an approximate lateral flow divide for shallow groundwater. No water enters or exits the model through the no flow boundaries.

Broad Sound, located at the central part of the northern model boundary and extending into the model domain, is defined as a constant head BC at an average tidal elevation of 2 mAHD. At the constant head BCs, the model calculates the flux of water that is required to either enter or exit the model to maintain the head at the set elevation at those cells.

(Blue: Constant Head BC; Orange: Drain BC; Grey: No Flow BC)

Figure 3-6 Model boundary conditions

3.4.4.2 Ephemeral creeks

Tooloombah and Deep Creeks are represented in the model using DRAIN BCs (refer [Figure 3-6](#page-28-0)). In a DRAIN cell, the model withdraws water when the calculated head in the cell exceeds the DRAIN elevation (or invert). For each model cell along the creeks, the DRAIN elevation has been derived from the elevation datasets used to define the ground elevation (refer Section [3.4.3\)](#page-23-0). As the model grid resolution (20 m to 640 m) is coarser than the DEM datasets (1m for the LIDAR, 10 m for DEM and 90m for SRTM), the invert for each model DRAIN cell was set at the minimal elevation of the overlaying DEM subset. The rate of drainage into or out of the watercourses (representing groundwater and surface water interaction) is controlled by a conductance factor. The conductance factor was selected to be large enough to prevent water from building up at the drain cell. The flow to the creek from groundwater is then controlled by the aquifer hydraulic properties and hydraulic gradients established between the simulated aquifer(s) and adjoining DRAIN cells.

Streamflow was simulated using the MODFLOW Recharge package, which is discussed below.

3.4.4.3 Groundwater recharge

Recharge is applied to the upper-most active layer of the model domain, over three zones. The recharge rate for each zone has been represented as a percentage of monthly rainfall and have been based on recharge estimates derived using the chloride mass balance method (see Section 10.5.6.6 of SEIS Chapter 10 – Groundwater). The respective recharge factors were determined during calibration with the constraint that recharge of the Alluvium is greater than the recharge over

Basement and the Flood recharge is higher than the Alluvium recharge. During calibration, the recharge of the Alluvium and Basement were bounded by the estimated values from field data (see Chapter 10 – Groundwater, Section 10.5.6.6). The percentage rates were used as calibration parameters, which are discussed in Section [3.5.](#page-30-0)

Groundwater recharge was applied to the model in three zones (refer [Figure 3-7\)](#page-29-0):

- Flood recharge located along Tooloombah and Deep Creeks to represent recharge following high streamflow events (15 mm/yr on average);
- Alluvium diffuse recharge located where Cenozoic (alluvial) sediments are present in the model (4.5 mm/yr); and
- Basement diffuse recharge located where Cenozoic (alluvial) sediments are absent (3 mm/yr).

3.4.4.4 Evapotranspiration

Excess groundwater recharge, which would otherwise result in the water table rising above ground surface, is removed during simulations by ET, which was modelled using the MODFLOW EVT package. ET occurs if the water table rises above a defined 'extinction depth', which in the Styx model is set equal to 3 m below ground surface. It does not include ET from the unsaturated zone, which is indirectly accounted for in the aquifer recharge rates (diffuse recharge).

The EVT package requires the definition of a maximum ET rate, which has been defined using the average monthly time series of potential evaporation.

Figure 3-7 Recharge zones

3.5 Model calibration

3.5.1 Overview

Model calibration generally involves changing values of model parameters within reasonable bounds until model outputs (e.g. heads or fluxes) fit historical measurements, such that the model can be accepted as a reasonable representation of the physical system of interest (Barnett et al. 2012). The calibration procedure followed these iterative steps:

- **•** Identify historical measurements of quantities that can be predicted by the model (calibration targets), which consists in this model of measurements of hydraulic head in groundwater bores;
- Identify the historical stresses on groundwater sources during the period when historical observations are available (rainfall recharge in this case);
- Run the groundwater model to simulate this historical period and compare the model predictions with the observations; and
- Adjust the model parameters within realistic/representative bounds and re-run the groundwater model until an acceptable statistical match is achieved between the simulated and observed values. These steps can be performed manually or automatically using parameter estimation software such as PEST. The model calibration reported here has been assisted by PEST followed by manual adjustment to select the parameter sets that provide the best fit with observed data as well as remaining consistent with the hydrogeological conceptualisation (further detail regarding the use of PEST is provided in sections [3.7.1](#page-89-1) and [3.7.2.2\)](#page-92-0) .

The model has been calibrated in steady state to determine initial heads for the transient model, and in transient mode for a period of 15 months.

3.5.2 Calibration targets

The model has been calibrated using hydraulic head data from 66 groundwater bores, described in Section 10.5.6.2 and Section 10.6.2.2 (SEIS Chapter 10 – Groundwater). [Figure 3-9](#page-32-0) presents a (premining) water table elevation contour plan that has been inferred and hand-drawn from the available data, which indicates regional groundwater flow towards the estuary and coast, and local groundwater flow to watercourses.

For 49 of the bores, the calibration targets consist of single hydraulic head observations. Timeseries data are available for 17 bores (refer [Figure 3-8\)](#page-31-0), primarily interpreted to screen the Alluvium and Styx Coal Measures. The hydrographs show:

- Generally, minor variation in Basement and Styx Coal Measures groundwater elevations is observed in likely response to rainfall; and
- Some variation in Alluvium groundwater elevations is observed (e.g. bores WMP08, WMP05 BH16 and BH01X) with up to 3 m difference in heads between wet and dry seasons. However, the observed rise in hydraulic head at WMP08 and WMP05 is likely related to stabilisation of water level post construction of the bores (suggesting low permeability).

[Table](#page-34-1) 3-3 lists the number of observation bores available for provision of head data for model calibration, separated by HSU. The hydraulic head targets range from 1.9 to 302 mAHD.

The location of the calibration target bores is shown in [Figure 3-10](#page-33-0).

Figure 3-8 Observation bore hydrographs by HSU

But as target location r2.mxd 12/5/201

Table 3-3 Observation bores per unit

3.5.3 Calibration results

Results of the model calibration are summarised in [Table](#page-34-2) 3-4 and [Table](#page-34-3) 3-5, representing the adopted values of hydraulic properties and recharge that are used later for the predictive simulations[. Figure 3-11](#page-35-1) shows a scattergram of observed verses simulated values of hydraulic head at the locations of the calibration targets, as well as the probability distribution for the calibration residuals (the differences between the observed and simulated values). The calibration residuals are presented on [Figure 3-13.](#page-37-0) Much of the calibration fit reflects the relationship between water table elevation and topography. The calibration fit is achieved as a balance between the applied rate of groundwater recharge and hydraulic conductivity of all units, particularly the outcropping units (the Cenozoic deposits, and the fractured and weathered zone of the outcropping basement and Styx Coal Measures). The calibration accounted for anticipated ranges of hydraulic conductivities based on field pumping tests and slug tests results (Section [2\)](#page-8-1), and results of testing completed elsewhere in the general area of the Project (see Section 10.5.6.3, SEIS Chapter 10 – Groundwater). For each HSU, a consistency with field geometrical mean results was favoured to marginal improvements of the calibration fit.

Table 3-4 Adopted hydraulic properties

Notes: 1 Kh = horizontal hydraulic conductivity, Kv = vertical hydraulic conductivity, Sy = Specific Yield

Table 3-5 Adopted Recharge

Figure 3-11 Calibration scattergram and residuals

The following presents a summary of calibration results and the adopted hydraulic properties:

- **The adopted hydraulic conductivity value of the Styx Coal Measures is at the lower end of the** expected range. This was required to obtain a suitable calibration to the observed hydraulic heads in the Styx Coal Measures, and to remain consistent with the overall conceptual understanding. The influence of this parameter on the model predictions is explored further in the uncertainty analysis (Sectio[n 3.7\)](#page-89-0).
- The calibrated diffuse rainfall recharge rates for the alluvium and basement are within the anticipated ranges. Variations in recharge rates are also explored as part of the uncertainty analysis (Section [3.7\)](#page-89-0).

The calibrated hydrographs are illustrated on [Figure 3-14.](#page-38-0) The predicted water levels at the observation bores show limited amplitude (i.e. change in level over time) which is not consistent with observed variability at some bores (BH01X and BH16) in response to rainfall recharge. The model is structurally limited in its ability to represent this behaviour, principally as a result of the manner in which recharge is simulated (i.e. it is expected most recharge occurs following high rainfall events, rather than as a percentage of all rainfall). The regional hydraulic gradient is well calibrated, providing confidence that there is a good balance between regional recharge and corresponding hydraulic conductivity. There is also good agreement between inferred pre-mine water table elevation contours [\(Figure 3-9\)](#page-32-0) and the model predicted pre-mine water table elevation contours [\(Figure 3-15\)](#page-41-0).

3.5.4 Verification

An additional 25 project monitoring bores were installed following the model calibration. The breakdown of the newly implemented observation bores by HSU is illustrated on table [Table](#page-36-2) 3-6. The newly acquired observations were imported into the calibrated model and a combined scatter plot was generated to assess how the new observations affect the overall calibration [\(Figure 3-12\)](#page-36-1). The sRMS with the new observations remains identical to the statistic on the calibration set (sRMS= 1.9%). Therefore, the new observations are consistent with the calibrated parameters for the model.
HSU	Model layer	No. of observation bores for calibration	No. of observation bores in the verification set	Total
Alluvium	1,2	29	b	34
Styx Coal	3,4,5	18	17	35
Measures				
Basement	-6	20		22

Table 3-6 Observation bores per unit

Figure 3-12 Calibration scattergram and residuals- verification

3.5.5 Model mass balance

The model water balance for the initial steady state period reported in [Table](#page-42-0) 3-7 is used to check whether the steady-state model is solving accurately and does not contain a mass-balance discrepancy. The percent error in the model mass balance is smaller than 0.002% and demonstrates the results are numerically accurate [\(Table](#page-42-0) 3-7). The transient mass balance for the calibration period is illustrated on [Figure 3-16](#page-42-1) and [Figure 3-17.](#page-42-2) It shows a large recharge spike following simulated rainfall events, which results in a rise in hydraulic heads. The increase in storage is balanced by removal of water from the model through ET and creek baseflow.

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Figure 3-14

Calibration hydrographs - Model Layer 1 and 2

 $Figure 3-14$ Calibration hydrographs - Model Layer 3 to 5

Figure 24c Calibration hydrographs - Model Layer 6

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Table 3-7 Steady state calibrated model water balance

Notes: 1 Positive values represent inflows to the model. Negative values represent flows out of the model. Values are totals across the model domain

Figure 3-16 Transient calibrated model mass balance - inflows

Figure 3-17 Transient calibrated model mass balance – outflows

3.6 Predictive simulations

3.6.1 Representation of mining

The mine development is represented in the groundwater modelling using the 18-year mine plan and schedule¹ provided by CQC, which is shown in [Figure 3-18.](#page-44-0) The mine schedule has been used to develop a passive dewatering sequence that is simulated by collecting groundwater reporting to the active pit areas (adopting an approach representative of sump pumping).

The dewatering schedule implemented in the model is based on the following assumptions:

- All groundwater inflow reporting to active mining areas will be collected in sumps and pumped out; and
- The mine pits will be progressively backfilled during mining.

Dewatering of mine pits and depressurisation at the pit faces is represented in the groundwater modelling using DRAIN BCs at which the hydraulic head is constrained to the pit face elevation (and the DRAIN conductance is set high enough $(640 \text{ m}^2/\text{d})$ so that flow to the cell is controlled by in-situ hydraulic properties rather than the ability of the drain to remove water). DRAIN BCs are activated sequentially according to the planned mine schedule as illustrated o[n Figure 3-18.](#page-44-0) When the DRAIN BCs are active, water discharges from the model to maintain the water table at the elevation of the pit floor. The water abstracted at the pit BCs over time is used to estimate the predicted mine dewatering rates.

3.6.1.1 Water storage dams

Five water storage dams are proposed during mining to manage and store water produced by the Project. The storage dams are not planned to be lined and therefore have the potential to leak water to the water table, thereby buffering the impact of dewatering on water table drawdown to some extent. The storage dams are not simulated in the model, which provides a conservative outcome in terms of potential water table drawdown during and following mining.

3.6.1.2 Pit backfill

The current mine plan includes progressive backfilling of the pits during the life of mine. The backfilling is represented in the model by removing the DRAIN BCs that represent mine dewatering after one year of simulated mining. The backfill material is assumed to have the same properties as the pre-mining material. This approximation has a limited influence on recovery of hydraulic heads in the backfilled pits, as the recovery is mainly controlled by regional aquifer properties.

3.6.1.3 Final mine-pit voids

All pits are backfilled to at least the pre-mine ground elevation and there will be no pit void following mine completion. Due to bulking of excavated materials, remnant waste rock stockpiles are likely to remain to some extent.

 1 The mining schedule has been modified during the course of this study. Both schedules (original and revised) were modelled in predictive mode and compared to each other. As the drawdown at end of mining was 2 % more conservative for the original schedule than for the revised schedule, the original schedule formed the basis for the modeling analysis. The revised schedule and associated predicted drawdowns are discussed in the uncertainty analysis section and illustrated on [Figure 3-66](#page-105-0) and [Figure 3-67](#page-106-0).

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3.6.1.4 Stress periods

Predictive model simulations are run for the 18 years of mining followed by a 500 year post-mining period. The stress period length is one month and the timestep length is 7 to 12 days (adopting 3 timesteps per stress period and a timestep multiplier of 1.2).

3.6.2 Prediction results

3.6.2.1 Groundwater heads and drawdown

The following presents a summary of predictive results of the calibrated groundwater model. The model incorporates the original (most conservative) proposed mining schedule and mine plan (ref. Mine Schedule – 20180406.shp) that has been provided by CQC. The mine plan and schedule is presented in [Figure 3-18.](#page-44-0) Model predicted pre-mine, during mining and post-mine groundwater heads and drawdown data are presented on the following figures:

- Water table (potentiometric) 'surface' and groundwater flow direction response (pre-mine, during mining and post-mine) – [Figure 3-19](#page-49-0) t[o Figure 3-25.](#page-55-0)
- Potentiometric surface 'drawdown' response (during mining and post-mine) [Figure 3-26](#page-56-0) to [Figure 3-32.](#page-62-0)
- Potentiometric surface drawdown at end of mining for each model layer showing the extent of desaturated HSUs for each layer– [Figure 3-33](#page-63-0) t[o Figure 3-37.](#page-67-0)
- Groundwater abstraction rates from mine pits (during mining) and cumulative life of mine groundwater production - [Figure 3-38.](#page-68-0)
- Proportional changes to baseflow and evapotranspiration losses for selected reaches of Tooloombah and Deep Creeks - [Figure 3-39](#page-68-1) an[d Figure 3-40.](#page-69-0)
- Hydrographs at locations of potential sensitive groundwater dependent ecosystems (GDEs) occurring with the predicted mine water affecting activities zone of influence [-Figure 3-42](#page-71-0) an[d Figure 3-43.](#page-72-0)
- Hydrographs at locations of potential sensitive third party groundwater users [Figure 3-45](#page-74-0) to [Figure 3-47.](#page-76-0)
- Potentiometric surface 'drawdown' response at the end of mining for an abstraction mitigation scenario for offsetting predicted drawdown impacts (discussed in section [3.6.2.4\)](#page-77-0) and a hydrograoh of the predicted available abstraction rate for a possible mitigation bore-[Figure 3-48](#page-77-1) an[d Figure 3-49.](#page-79-0)
- Maximum predicted drawdown extent in relation to potential ASS [Figure 3-50.](#page-80-0)
- [Figure 3-51](#page-81-0) t[o Figure 3-53](#page-83-0) predicted cross-sectional drawdown response for each model layer for pre-mine, year 12 (maximum pit depth) and at end of mining.

The following provides discussion around the changes predicted for the water table surface and shallow groundwater flow directions [\(Figure 3-19](#page-49-0) to [Figure 3-25\)](#page-55-0) in response to mining water affecting activities:

- **•** The predicted water table elevation contours are a reasonable representation of the inferred water table elevation contours presented i[n Figure 3-9](#page-32-0) (manually drawn based on available standing water level data for the broader study area).
- By mining year 5 through to end of mining, pit dewatering is predicted to capture groundwater from within the proposed lease area and to some extent from the mid- to lower catchment areas of Tooloombah and Deep Creeks. However, away from the proposed mine

lease area, water table elevation contours are predicted to remain relatively consistent with the pre-mine condition.

- During the early recovery phase, the year 10 post-mining water table elevation contours are predicted to remain much the same as the end of mining contours but by year 25 post-mining the effects of recovery can begin to be seen. During the post-mining recovery phase, water table elevation contours outside the proposed mine lease area are predicted to remain relatively consistent with the pre-mine condition.
- At all times during the simulated mining and recovery phases water table contours downstream of the Tooloombah and Deep Creek confluence remain very consistent with the pre-mine condition, indicating the water affecting activities of the proposed mine will not impact on groundwater quantity or groundwater-surface water interactions below the confluence.

The following provides a discussion of the changes predicted for the potentiometric surface and the saturated extent of HSU1 (alluvium) and HSU2 (the over-, inter- and under-burden units of the Styx Coal Measures) in response to mining activities, which are presented in [Figure](#page-56-0) 3-26 t[o Figure 3-37:](#page-67-0)

- **E** Mining at the 'Open Cut 2' pit, by year 5
	- 1 m drawdown contour extends to Tooloombah and Deep Creeks on the western and eastern boundaries of ML80187; and
	- 0.1 m drawdown contour (adopted as the zone of influence) extends beyond Tooloombah Creek to the northeast, but not to the confluence of the two creeks.
- Mining at both the 'Open Cut 1' and 'Open Cut 2' pits, by year 10 through end of mining
	- 1 m drawdown contour extends to and beyond Tooloombah and Deep Creeks on the western and eastern boundaries of ML80187, as well as further to the south and north of the ML, to intersect stream reaches of the mid- to lower Deep Creek and mid-Tooloombah Creek
	- 0.1 m drawdown contour extends further beyond Tooloombah Creek to the northeast and within around 1,500 to 2,000 m of Styx River, but not to the confluence of the two creeks
	- alluvium (HSU1) is dewatered over much of the central portion of ML 80187, with small areas outside the ML also dewatered [\(Figure 3-33\)](#page-63-0)
	- overburden coal measures (HSU3) is dewatered over the central portion of ML 80187 [\(Figure 3-34\)](#page-64-0)
	- interburden coal measures (HSU3) is dewatered in the areas of 'Open Cut 1' and 'Open Cut 2' ([Figure 3-35\)](#page-65-0)
	- underburden coal measures (HSU3) remains saturated beneath ML 80187 [\(Figure](#page-66-0) [3-36\)](#page-66-0)
- By year 10 through 25 into closure (with all voids backfilled) the predicted extent of the
	- 1 m drawdown contour remains similar to the contour predicted at the end of mining, but extends further east into the Deep Creek catchment and south into both the Deep and Tooloombah Creek catchments
	- 0.1 m drawdown contour extent remains similar to the contour predicted at the end of mining

By year 50 into closure the predicted extent of the 0.1 and 1 m drawdown contours has begun to shrink back towards the decommissioned and back filled pits, and by year 100 into closure the groundwater system is predicted to have fully recovered.

3.6.2.2 Dewatering rates

Model predicted dewatering rates presented on [Figure 3-38](#page-68-0) indicate the peak dewatering rate of around 640 ML/yr will be reached in year 10, rising from around 340 ML/yr at commencement of mining and declining to less than 50 ML/yr at completion of mining. The cumulative abstraction over the life of mine is predicted to be around 5,500 ML.

3.6.2.3 Direct effects at sensitive environmental receptors

A key direct effect of mining on groundwater is drawdown associated with dewatering and the associated indirect effects of reduced baseflow to support in-stream GDEs and riparian GDE access to the water table [\(Figure 3-41\)](#page-70-0)[. Figure 3-39](#page-68-1) an[d Figure 3-40](#page-69-0) present model predicted changes in flux (baseflow and ET) to the riparian zones of Tooloombah and Deep Creeks in response to mining activities as a proportion of the predicted (no mine) basecase, showing:

- Tooloombah Creek
	- little to no change in flux between the 'no mine' and 'mining' scenario for the upper reach (above Bruce Highway); and
	- upwards of 40% reduction in flux between the 'no mine' and 'mining' scenario for the lower reach (below Bruce Highway), with flux slowly returning to background after closure (\approx 50% recovery by around 65 years after closure, and the remaining \approx 50% occurring within another 20 years or so).
- Deep Creek
	- less than 15% reduction in flux between the 'no mine' and 'mining' scenario for the upper reach (above WMP10; [Figure 3-10](#page-33-0)), with flux slowly returning to background within around 75 years after closure;
	- 60% reduction in flux between the 'no mine' and 'mining' scenario for the middle reach (between WMP10 and the confluence with the tributary creek that runs through ML 80187 [\(Figure 3-10\)](#page-33-0), with flux slowly returning to background after closure \sim 25% recovery by around 60 years after closure, and the remaining \sim 75% occurring within another 20 years or so); and
	- less than 15% reduction in flux between the 'no mine' and 'mining' scenario for the lower reach (from the confluence with the tributary creek that runs through ML 80187and the confluence of Deep and Tooloombah Creeks), with flux slowly returning to background within around 75 years after closure

Model predicted hydrographs at selected locations where potential GDEs occur are presented in [Figure 3-42](#page-71-0) and [Figure 3-43,](#page-72-0) and show:

Drawdown at the location where stygofauna have been identified in Deep Creek catchment alluvium (bore STX093[; Figure 3-26](#page-56-0) and [Figure 3-42\)](#page-71-0) in response to mining activities is predicted to result in an almost 90% loss of vertical habitat over the life of mining after which full recovery occurs by around 50 years after closure (50% recovery occurring by around 15 years after closure).

• Drawdown at the location of a potential Type 3 GDEs (WMP25 and WMP27[; Figure 3-10a](#page-33-0)nd [Figure 3-43\)](#page-72-0) in response to mining activities is predicted to be less than 2 m in an area where the water table has been gauged at around 10 m or more.

Model predicted hydrographs at third-party bores [\(Figure 3-44\)](#page-73-0) located in the Project area are presented in [Figure 3-45](#page-74-0) to [Figure 3-47,](#page-76-0) and show:

- **Drawdown in response to mining activities at the location of BH28 and BH28a of up to around** 2 m is predicted over the life of mine and out to 70 years after closure.
- **Drawdown in response to mining activities is unlikely to occur at other third-party bores in** the area.

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Figure 3-38 Predicted groundwater abstractions from mine (North and South Pits)

Figure 3-39 Predicted impact on baseflow and evapotranspiration (Tooloombah Creek)

Figure 3-40 Predicted impact on baseflow and evapotranspiration (Deep Creek)

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Figure 3-42 Hydrograph showing predicted transient water table response to mine water affecting activities at STX 093 (stygofauna bore, Type 1 GDE)

Figure 3-43 Hydrograph showing predicted transient water table response to mine water affecting activities at potential Type 3 GDEs on western ML 80187 boundary (WMP25 and WMP27)

Figure 3-45 Hydrograph showing predicted transient water table response to mine water affecting activities at BH28A and BH28 (third party water user bores)

Figure 3-46 Hydrograph showing predicted transient water table response to mine water affecting activities at BH01X, BH16 and BH20 (third party water user bores)

Figure 3-47 Hydrograph showing predicted transient water table response to mine water affecting activities at BH04 (third party water user bore)

3.6.2.4 Abstraction as mitigation measure to sustain permanent pools

The model was used to undertake a preliminary assessment of the feasibility of implementing abstraction bores to provide water to the permanent pools of Tooloombah and Deep Creeks during dry seasons, to mitigate the impact of drawdown associated with mining. The model tested six abstraction bores in the Styx Coal Measure interburden unit by setting a drain cells at the base of the interburden layer, active from the start of mining to 20 years post mining operation covering a period of 38 years. While the water demand for supplementing pools will likely only be required during the dry season, the scenario assumed continuous abstraction, providing an estimate of longterm sustainable yield. The average abstraction rate per bore for the 38 years of dewatering period was 0.7L/s with a minimum rate of 0.55L/s after 38 years [\(Figure 3-48\)](#page-77-0). The drawdown resulting at the end of the mining period is illustrated on [Figure 3-49](#page-79-0) which shows only minor differences to the basecase predicted drawdown [\(Figure 3-28\)](#page-58-0). This preliminary assessment suggests that the Styx Coal Measures may be a viable source of water for mitigating the impact to pools due to drawdown, although further detailed assessment would be required to account for the seasonal water demand at each pool and drawdown at the pumping bore, and further work required to reduce uncertainty associated with model predicted rates, integrate flux targets in the model calibration to better constrain the flux estimates and reduce uncertainty.

Figure 3-48 Predicted available abstraction rate for mitigation bore

3.6.2.5 Acid Sulfate Soil Interaction

[Figure 3-50](#page-80-0) presents a map showing the spatial distribution of ASS potential overlain with maximum model predicted drawdown contours. The figure shows there is low to extremely low probability of ASS in the Project area. Also plotted on [Figure 3-50](#page-80-0) are mineral exploration holes where predominantly Non-acid Forming (NAF) materials (less than 10% Potentially Acid Forming (PAF) materials) have been identified, which is consistent with the mapping undertaken by CSIRO (2011). Note that the predominantly NAF materials are logged as occurring more than 15 m below ground surface in the Styx Coal Measures.

The hydrographs presented on [Figure 3-50](#page-80-0) show:

- Outside the ML (one location), PAF materials occur more than 40 m below the water table at all times during and following mining.
- The full drawdown intersection at one location (STX145C, within Open Cut 2) might expose some material having a low probability of PAF material, but this material will be mined.
- Some exposure of low probability PAF material may occur very close to the northern limit of Open Cut 2 pit (STX136C) due to drawdown.

3.6.2.6 Groundwater system drawdown response in cross-section

[Figure 3-51](#page-81-0) through [Figure 3-53](#page-83-0) present a west-east aligned cross-section through ML 801887 showing the model predicted pre-mine, year 12 and at end of mining, potentiometric surfaces. [Figure](#page-84-0) 3-54 t[o Figure 3-56](#page-86-0) present the same information for a south-north aligned cross-section through ML 801887. The cross-sections and predicted potentiometric surfaces show:

- The pre-mine potentiometric surfaces for each HSU are essentially the same, with the potential for slightly higher heads in the Styx Coal Measures where they subcrop or outcrop on the western side of the geological basin.
- During mining, with the exception of the alluvium (HSU1) and the overburden coal measures (HSU2) that become unsaturated around the mine pits
	- the lateral extent of the drawn-down potentiometric surfaces are similar, with the basement (HSU3) zone of influence being slightly larger (likely the result of a lower storage coefficient); and
	- the vertical depths differ by many 10s of metres toward the end of mining possibly the result of targeted dewatering and recovery occurring from bottom up.
- The basement is not dewatered during mining but is depressurised.

3.6.2.7 Seawater-Freshwater Interface Interaction

[Figure 3-55](#page-85-0) and [Figure 3-56](#page-86-0) present south-north aligned cross-sections through ML 80187 that show the HSUs and the model predicted potentiometric surfaces for each of the HSUs at years 12 and 18. Also, overlain on these cross-sections is the model predicted pre-mine water table surface. The cross-sections show that at the most northerly extent (around the upper reach of Styx River) there is unlikely to be any measurable drawdown in response to mine dewatering that can induce inland mobilisation of the seawater-'freshwater' interface, whether it be located near the point of discharge of Styx River into the Broad Sound estuary or closer to the coast at Broad Sound.

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pre-mine potentiometric surface (west to east through ML)

Cross-section showing model predicted drawdown (west to ^east through ML) – year 12 (maximum pit depth)

Cross-section showing model predicted drawdown (west to east through ML) – at end of mining

Figure 3-55

Cross-section showing model predicted drawdown (south to north through ML) – year 12 (maximum pit depth)

Figure 3-56

Cross-section showing model predicted drawdown (south to north through ML) – at end of mining

3.6.3 Conservative approach to modelling

3.6.3.1 Details

Models are simplified representations of reality, consequently their predictions are always uncertain. A conservative approach to modelling involves adopting parameters that will provide an overestimation of predicted impacts. In the Styx mine environment, the key impacts to existing users (environmental, social and economic) will arise in response to water table drawdown, depressurisation of deeper (confined) hydrostratigraphic units (HSUs) and reduced baseflow to water courses (quantity and temporal) in response to mine water management. The impacts could include:

- Reduced access to groundwater by riparian and terrestrial GDEs;
- **Exposure of acid sulphate soils to the atmosphere; and**
- Inland mobilisation of the seawater-freshwater interface.

Conservatism in the case of the Styx model is a bias toward an overestimation of the expected 'cone' of drawdown in response to mining activities. The features of the modelling approach that provide conservatism to the results are summarised in [Table](#page-87-0) 3-8.

Process	Model representation	Conservative aspect
HSU hydraulic parameters	During the uncertainty analysis, conservative (high) values of regional hydraulic conductivities in all HSUs have been simulated. It was shown that the predictive uncertainty is primarily carried by the coal seams and interburden HSU. By adopting conservative hydraulic conductivities, it is predicted that drawdown remains less than 0.1 m above the confluence of Tooloombah and Deep Creeks (i.e. the 0.1 m drawdown contour does not extend to Styx River, Broad Sound estuary or the coast).	Assumed range in hydraulic conductivity values for the uncertainty analysis: spanned at least 1 order of magnitude beyond best calibrated values; extended at least up to the maximum п value of field estimate for each HSU (from slug and pumping tests).
Mining operation	Two mining schedules have been modelled to test groundwater system sensitivity to pit development and backfilling schedules.	The most conservative of the two schedules has been represented for the predictive analysis.
Backfill moisture content	Backfill materials are assumed to be completely dry.	Backfill materials will have some moisture content, I.e. will not be entirely dry. Not simulating this 'starting point' means that predicted timeframes for groundwater recovery will be over-estimated.
Storage coefficient	Storage parameters (S and Sy) have been represented at very low values.	Low storage values will result in an over- estimate of drawdown extent when compared to higher values, however will result in an underestimation of recovery.

Table 3-8 Summary details of conservative aspects of the Project numerical groundwater flow model

3.6.3.2 Parsimony versus over-parameterization (or uniform hydraulic conductivity versus heterogeneous)

In the model, each HSU is represented by a single value of hydraulic conductivity uniformly distributed along the whole HSU. As the number of parameters is kept to a conceptually meaningful minimum and, as there are more observations (head targets) than parameters, this method fits the "parsimony approach".

An alternative to this approach is to allow different values of hydraulic conductivity to spatially characterise each HSU. This approach allows some heterogeneity to be represented within each HSU. As the number of parameters in this approach generally becomes greater than the number of observations this approach is called the "over-parameterised approach".

In a regional aquifer, heads are controlled by the distribution of hydraulic properties within the aquifer. The distribution is complex and often indeterminate with limited field observations. The parsimony approach considers the regional aquifer to be represented by a uniform regional value of each hydraulic parameter (Kh, Kv, Sy, Ss), and local variation from the regional value can be described as "noise from the main signal".

Overly simplistic models fail to capture the signal (i.e. they underfit the data and provide poor prediction). Overly complex models can hide the noise from the regional signal and, by overfitting, the data can provide for poor predictions. A decent parsimony model should derive a good fit with observation and achieve an unbiased normally distributed residual (i.e. the difference between observed and calculated values) of around zero, with a spatially randomly distributed residual. This is what is observed for the Styx model.

With an over-parameterized approach, a better calibration (at the cost of a more important computational effort) can be achieved as locally defined parameters can adjust to local observations. However, a better calibrated model doesn't make a better predictor as many parameters remain indeterminate. Those parameters are not constrained by observation and their value is only determined by pre-calibration consideration. Therefore, such an approach is only relevant with the

implementation of a thorough uncertainty analysis (via a null space uncertainty analysis) that is demanding in terms of cost and computational effort. The null-space uncertainty analysis provides a sophisticated and refined uncertainty approach that maintains good compliance with observations. However, the uncertainty analysis for a parsimony approach can be designed to encompass the uncertainty that would be revealed in the over-parameterized approach. In other words, with conservative criteria such as those applied for the Styx model (i.e. "stretching" regional parameters to conservatively high values) the range of predictions (from minimum to maximum drawdown) are wider than the range of predictions that would arise from the null space uncertainty analysis. Over-parameterisation is more relevant when it becomes necessary to refine more precisely the range of predictions at any point.

3.7 Sensitivity and uncertainty analysis

3.7.1 Calibration sensitivity

During the automatic calibration, parameter estimation software (PEST; Doherty, 2016) calculates the sensitivity of calibration for each parameter. This allows the user to visualise the relative importance of each parameter in the calibration process. The most sensitive parameters have a stronger influence on calibration results and their value is more constrained by the calibration process. The least sensitive parameters have a lower influence on the calibration and their value is only loosely determined by the calibration. The parameter sensitivity results are illustrated in [Table](#page-89-0) 3-9. To explore further the sensitivity of model parameters, a series of models have been run that changes a single parameter at a time from its calibrated value. For each of those runs, the sum of squared residual (Phi) has been calculated. Phi is an indication of the calibration fitness - the lower the value the better is the match between observed and calculated values. The range of parameters used for the sensitivity runs are summarised on [Table](#page-90-0) 3-10 and the results are illustrated on [Figure](#page-91-0) 3-57.

Parameter ¹	HSU	Sensitivity ²
Kh, Kv	Underburden	0.370
Kh, Kv	Alluvium	0.309
Recharge	Recharge	0.226
Kh, Kv	Basement	0.188
Kh, Kv	Weathered	0.076
Kh, Kv	Overburden	0.026
Sy	Alluvium	0.010
Kh, Kv	Coal and interburden	0.006
Sy	Weathered basement	0.002

Table 3-9 Model calibration parameter sensitivity (sorted by most to least sensitive)

Note: 1. Kh – horizontal hydraulic conductivity; Kv – vertical hydraulic conductivity; Sy – Specific yield 2 This is a unitless qualitative measure of model sensitivity to changes in parameters (Doherty, 2016)

The three most sensitive parameters controlling model calibration are shown to be:

- 1. Hydraulic conductivity (Kh, Kv) of the Styx Coal Measures underburden;
- 2. Hydraulic conductivity (Kh, Kv) of the alluvium; and
- 3. Recharge rate(s).

The parameters that are least sensitive (in terms of model calibration) are specific yield of the alluvium and weathered basement aquifers, drain conductance, evaporation extinction depth and hydraulic conductivity of the coal seams and interburden layer (#3).

Table 3-10 Sensitivity runs

Note: 1. Kv is an order of magnitude lower than Kh

2. Kv is equal to Kh.

3. Not part of calibration process, therefore included in sensitivity assessment

3.7.2 Predictive Uncertainty

3.7.2.1 Objective

Model calibration aims to optimise the match between field observations and their calculated equivalent. All the calibrated model parameter values emerge from the constraints the target observations apply to the model during calibration. For this reason, a single calibrated model offers the best estimate of predictive impacts based on those observations. However, due to the noise in the observation data and to some hydrogeological features that are not represented in the model, the calibrated model may not be the most accurate predictor of the real system. To explore alternatives to the calibrated model (and the hydrogeological conceptualisation), a predictive uncertainty analysis has been conducted. The aim of the uncertainty analysis is to explore the variability in the model prediction that is consistent with the target observations used to calibrate the model. The predictive uncertainty arises from parameters with low sensitivity and from correlation between parameters (meaning that recombining parameters according to their correlation doesn't affect the calibration). The different approaches detailed in this section explore both sources of uncertainty.

Figure 3-57 Sensitivity run results

3.7.2.2 Method

Non-uniqueness

As there is often non-uniqueness in relation to combinations of simulated hydraulic properties that achieve an acceptable calibration, the model has been tested for predictive uncertainty relating to different combinations of hydraulic properties that are varied within acceptable ranges based on observations of the simulated groundwater system, results of aquifer testing and the literature. The predictive uncertainty aims to explore possible range in predictive outcomes whilst maintaining acceptable calibration.

A total of 190 predictive models were generated and assessed. The following techniques were used to generate the uncertainty models:

- **•** From the calibration parameter sensitivity assessment, the PEST Null Space Monte Carlo technique (Doherty 2016) has been used to define the calibration null space, which identifies combination of parameters that have limited influence on the calibration (i.e. acceptable model calibration). Of the possible combinations identified, the models were reviewed to assess those models with parameters that remained consistent with the hydrogeological conceptualisation. Of the original 129 simulations, 106 models were selected and used for the predictive uncertainty analysis.
- **•** Recharge / Kh correlation The model calibration identified that estimated hydraulic heads have a strong correlation between recharge rate and hydraulic conductivity, as would be expected. It is possible to generate models that maintain acceptable calibration by varying recharge and hydraulic conductivity by a similar factor. Based on this, 100 models were generated and run to assess this observed correlation. Of the 100 models, 23 maintained acceptable calibration and were used to assess the effect on the predictive results. During calibration, the ratio between Kh and Kv was maintained constant for each HSU. The ratio was set to 10 for the quaternary alluvial deposits and weathered basement and to 1 for all other units (Styx Coal Measures and unweathered basement). The sensitivity of the Kh/Kv ratio on the predicted drawdown was assessed by running a scenario assuming a ratio of 10 and 100 for the Styx Coal Measures (see Section [3.7.2.3](#page-94-0) and [Figure 3-65.](#page-104-0)
- **•** Based on model sensitivity results, model hydraulic conductivities within the Styx basin were 'stretched' to assess the breakpoint where drawdown becomes significative downstream at the confluence of the Tooloombah creek and Deep creek, and at the coast. Those scenarios allow an assessment of what conditions are required for significant impact to appear. Those conditions are then compared to the literature, field observations and calibration constraints to assess their eventuality. Those scenarios are reported in the following Section.

The distribution of the calibrated parameters assessed in the uncertainty analysis is illustrated in [Figure 3-58](#page-93-0) and [Figure 3-59.](#page-93-1)

whiskers= minimum and maximum, box= 25th to 75th percentile, line within box= median, cross= average, points= outliers

whiskers= minimum and maximum, box= $25th$ to 75th percentile, line within box= median, cross= average, points= outliers

Figure 3-59 Box and Whisker plot showing range of average annual recharge rates assessed as part of uncertainty analysis

Mine schedule

The uncertainty related to the mining schedule was assessed by running the predictive model with a revised mining schedule to compare with the original schedule predictions.

Climate variability

The uncertainty related to future climate conditions, including both a drier and a wetter climate over the period of mining. Four scenarios were simulated:

• During the driest period in the historical rainfall record (between 1990 to 2009) total rainfall was around 15% less than the average recorded at Rockhampton (station 039083) and Strathmuir (station 033189). It is likely that recharge rates do not have a linear relationship

with rainfall rates, and a decline of 15% in rainfall will possibly give rise to greater rates of decline in recharge. To represent this situation, three 'drought' scenarios have been simulated for recharge - 15%, 30% and 60% reduction - during the life of mine, with climate conditions returning to average during the post mining period; and

During the wettest period in the historical rainfall record (between 1949 to 1954), rainfall was around 15% higher than the average. To represent this situation, 15% more rainfall recharge has been simulated for a period of 5 years commencing from the start of mining activities.

Backfilling and hydraulic loading

The effect of stockpiling and above ground waste storage, backfilling the pits during mining operations, and the existence of residual waste landforms after closure has been assessed by conservatively assuming (in the absence of data):

- K and Sy of backfill material hydraulic are 0.02 m/d and 0.05, respectively (see [Table](#page-34-0) 3-4 for comparison); and
- K and Sy of the alluvium beneath the landforms are reduced by 50% due to compression to 2 m/d and 0.005, respectively; see [Table](#page-34-0) 3-4 for comparison).

3.7.2.3 Results

Non-uniqueness

The results of each predictive uncertainty model have been reviewed, and the following models have been selected for discussion:

• Predictive uncertainty models that result in the lowest and largest mine dewatering rates [\(Figure 3-60\)](#page-95-0). The associated parameters for the uncertainty models that predicted the lowest and highest groundwater pit dewatering volumes (around 180 and 1,300 ML/yr, respectively) are presented, along with the base case parameters for comparison, in [Table](#page-95-1) 3-11. The model with the maximum predicted inflows has higher hydraulic conductivity values in all modelled HSUs (greater than half an order of magnitude) and recharge is approximately five times greater than that of the base case model, which is considered unrealistic. As expected, the model with the minimum predicted inflows has lower hydraulic conductivity values in all modelled HSUs and recharge rates five times less than that of the base case model, which is again considered unrealistic.

Figure 3-60 Groundwater abstraction from pits – predictive uncertainty results

Predicted water table drawdowns from the uncertainty analysis having the greatest drawdown extent at the end of mining and 10-years post-mining are presented o[n Figure 3-61](#page-97-0) an[d Figure 3-62.](#page-98-0) The following presents a summary of the outcomes, when compared to the basecase predictive model results [\(Figure 3-21](#page-51-0) an[d Figure 3-22\)](#page-52-0):

- **•** The northern extent of the 0.1 m drawdown contour (the zone of impact extent) remains unchanged, but it extends closer to the Styx River (within 250 m at the end of mining and within 100 m at 10-years post-mining);
- **·** The 1 m drawdown contour is still constrained largely to the east of Tooloombah Creek.; and
- The predicted drawdown remains unchanged at BH28A/BH28 (third party user) and the Type 3 GDE on western ML 80187 (shown as GDE6 on [Figure 3-61](#page-97-0) and [Figure 3-62\)](#page-98-0).

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SA\MXD\FINAL\1000111_WT_DDN_10yrs post-mining - UNCERTAINTY.mxd 12/20/201

- **•** The predicted water table drawdown at WMP28 (next to Tooloombah Creek, and close to the western boundary of ML 80187) is predicted to increase by a maximum of 4m during mining operations. By the end of mining operation the difference of drawdown at WMP28 has reduced to about 2m.
- The predicted water table drawdown at Deep Creek on the eastern boundary of ML 80187 is predicted to remain unchanged.
- Predictive uncertainty analysis indicates drawdown associated with mine water affecting activities will not extend to downstream areas identified with potential for acid sulphate soils or further north along the Styx River (indicating the conclusion from the adopted calibrated model that the potential for inland mobilisation of the seawater interface, if it were beneath Styx River remains valid, i.e. the potential for mobilisation is low / negligible).

[Figure](#page-100-0) 3-63 and [Figure 3-64,](#page-101-0) present predicted pre-mine and 10 years post-mine water table elevation contours for the 'most severe possible' scenario discussed above, and shows that downstream of the confluence of Tooloombah and Deep Creeks, the water table contours remain unchanged from the pre-mine condition.

The plausible parameter set that produces the 'most severe possible' model predictions in terms of groundwater head drawdown are presented on [Table](#page-99-0) 3-12. The most significant change in parameters is in the modelled recharge, which is around two times less than that in the base case prediction model.

Parameter	HSU	Base case calibrated value	Uncertainty value
Kh (m/d)	Alluvium	4.1	4.1
Kh (m/d)	Styx Coal Measures -overburden	2.0×10^{-2}	2×10^{-3}
Kh (m/d)	Styx Coal Measures - Coal seams and interburden	3.0×10^{-3}	1.8×10^{-2}
Kh (m/d)	Styx Coal Measures -underburden	4.0×10^{-3}	1.1×10^{-3}
Kh (m/d)	Weathered basement	1.0	1.0
Kh (m/d)	Basement	4×10^{-4}	4.4×10^{-4}
Kv (m/d)	Alluvium	0.41	0.41
Kv (m/d)	Styx Coal Measures -overburden	2.0×10^{-2}	2×10^{-3}
Kv (m/d)	Styx Coal Measures - coal seams and interburden	3.0×10^{-3}	1.8×10^{-2}
Kv (m/d)	Styx Coal Measures -underburden	4.0×10^{-3}	1.1×10^{-3}
Kv (m/d)	Weathered basement	0.1	0.1
Kv (m/d)	Basement	4×10^{-4}	4.4×10^{-4}
Alluvium diffuse recharge (mm/yr)	Alluvium	4.5	4.5
Flood recharge (mm/yr)	Creeks / alluvium	15	15
Weathered basement diffuse recharge (mm/yr)	Basement	3	3
$Sy (-)$	Alluvium	0.01	0.01
$Sy (-)$	Weathered basement	0.005	0.005

Table 3-12 Model parameters used in the 'worst-case' water table drawdown uncertainty analyses

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BRUGGEMANNGD B:\1000111_Styx_SEIS_post-submission\GIS\DATA\MXD\FINAL\1000111 WT wcs_base case-worst case __10yrs post m ining R2.m xd __12/20/2018

With a Kh/Kv ratio of 10 or 100 for the Styx Coal Measures [\(Figure 3-65\)](#page-104-0), the variation in terms of predicted (spatial) drawdown is not significant and tends to show the isotropic condition (Kh/Kv =1, as adopted in the basecase) offers a more conservative approach, indicating the extent of drawdown is mainly controlled by values of Kh and storage coefficients in the Styx Coal Measures.

The 0.1m drawdown contour for the Kh/Kv ratio of 100 scenario shows a combination of effects due to mining and due to the numerical accuracy of the model solutions - noting drawdown is calculated as the difference between a reference simulation without mining and a predictive simulation with mining. Specifically, the areas of 'satellite' drawdown beyond the main area of mining-related drawdown are likely to reflect the limits of accuracy of the simulations. On this basis, the 0.1-m contours of drawdown should be interpreted as indicative results that include numerical error of similar order of magnitude to the drawdown.

Mine schedule

[Figure 3-66](#page-105-0) presents the revised mine layout and schedule for the Project (open cut 2 scenario; compare with the original mine layout and schedule i[n Figure 3-18\)](#page-44-0). As shown the annual sequences of the revised schedule between 2019 and 2029 have a north-south direction while those sequences where in east-west direction in the original schedule. The life of the mine is very similar for both schedules and the differences are detailed in [Table](#page-102-0) 3-13. The difference between the two schedules at the end of 2036 (after 18 years of mining) is very similar as illustrated on [Figure 3-67.](#page-106-0) An integration of the volume of rock dewatered indicates the original schedule has 2% more drawdown than the revised schedule.

Table 3-13 Comparison of original and revised mining schedule

Climate variability

Under drier climate conditions [\(Figure 3-68](#page-107-0) t[o Figure 3-70\)](#page-109-0), there is a predicted 'general' drawdown across the entire model domain of between 0.1 to more than 2 m that is attributable to lower rates of recharge with progressively more drawdown predicted for progressively drier conditions (i.e. moving from 15% less recharge through to 30% and 60% less recharge), as would be expected. However, the model predicted 5 m drawdown remains relatively unchanged between the different drought scenarios when compared against the base case mining / average rainfall condition indicating the additional 'cumulative' effect of mine dewatering is negligible.

For above average recharge conditions [\(Figure 3-71\)](#page-110-0), there is basically no change predicted from basecase mining average recharge scenario, except isolated areas where drawup is predicted.

Backfilling and hydraulic loading

The spatial distribution of, and altered (lower K and Sy) hydraulic properties of the alluvium beneath stockpiles, waste landforms combined with the altered (higher K and Sy) hydraulic properties of backfill materials results in a subtle NW-SE elongation of the predicted zone of influence, with the 1m drawdown contour passing through Tooloombah Creek [\(Figure 3-72\)](#page-111-0). However the extent of drawdown (as indicated by the 0.1m contour) remains similar to the base case (which simulates no backfill or hydraulic loading) and emphasises the extent of the drawdown cone is mainly controlled by the properties of the coal seams and interburden.

This scenario can be considered conservative, as it assumes the backfill material is dry when it returns to the pit, although in reality the backfill material will have some moisture content. .

3.7.3 Impact breaking-point assessment

3.7.3.1 Methodology

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The uncertainty analysis detailed in Section 3.7.2 explores various combinations of parameters that maintains model calibration. From those calibrated sets, an assessment of whether noticeable impact arises has been conducted. As reported, no parameter set triggered significant drawdown near the coast or at the confluence of Tooloombah and Deep Creeks (i.e. just upstream of the tidal zone) indicating the likelihood of impact to hydrology downstream of the confluence of Tooloombah and Deep Creek is relatively small.

To complement the uncertainty analysis reported in the previous chapter, the approach reported in this chapter explores the condition necessary to trigger significant drawdown downstream of the confluence of Tooloombah and Deep Creeks. It is likely that such drawdown would pose a risk to environmental assets (e.g. by potentially triggering sea water intrusion and exposure of ASS). This 'breaking-point' assessment provides the opportunity to test the accepted hydrogeological conceptualisation.

Two observation points (as illustrated on [Figure](#page-112-0) 3-73) are used as a proxy to demonstrating the possible evolution of the cone of water table drawdown (depression) for the 'breaking-point' assessment. It should be noted that the coast is represented in the model by a constant head BC. To prevent the constant head BC to fully control the head of the observation point, it was located 300m further south.

For this analysis, diffuse rainfall recharge was maintained at the calibrated value. It has already been demonstrated (see previous section) that higher hydraulic conductivities paired with higher recharge rates have only limited influence on the depression cone extent compared to the basecase. Consequently, to trigger larger drawdown, it is necessary to make the assessment with a recharge rate that does not change by the same ratio as hydraulic conductivity (in this case, recharge remains the same as the basecase)2.

Conceptually, it is understood that, for a given recharge rate, a wider drawdown cone would be observed for hydraulic conductivities higher than the calibrated conductivities. For this analysis, only hydraulic conductivities higher than the calibrated value are considered.

 $²$ As recharge and hydraulic conductivity have a 1:1 correlation, it would be possible to extrapolate this</sup> analysis to a different value of recharge by applying the same multiplier on the value of hydraulic conductivities considered in this analysis

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00111_Styx_SEIS_post-submission\GIS\DATA\MXD\FINAL\1000111 A6 DD comparison_original_new mine schedule.mxd 12/20/2018

DATA SOURCE QLD Open Source Data, 2018; Waratah Coal, 2018; Geofabric v2.1, Bureau of Meteorology, 2012

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QLD Open Source Data, 2018

BRUGGEMANNGD B:\1000111_Styx_SEIS_post-submission\GIS\DATA\MXD\FINAL\1000111 A6 WT wet scenarios 15p more recharge during LOM r4.mxd 12/20/2018

QLD Open Source Data, 2018

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Figure 3-73 Location of observations for the analysis

Seven scenarios have been designed and run. The description of the scenarios and justification are as follow:

Scenario 1

For this scenario the hydraulic conductivity of the Styx overburden alone was progressively increased. The sensitivity analysis [\(Figure](#page-91-0) 3-57) showed this unit has limited influence on the calibration. The low sensitivity indicates the parameter value is not well constrained by the calibration and hence can potentially be higher. In particular, the calibrated value for this parameter (0.0017 m/d) is located at the lower end of the expected range based on slug test estimates (0.0005 to 0.05 m/d).

Scenario 2

For this scenario the hydraulic conductivity of the Styx interburden alone was increased. The interburden hydraulic conductivity is the least sensitive parameter of the calibration [\(Figure](#page-91-0) 3-57) and therefore its value is not well constrained by the calibration. However, as the interburden constitutes the targeted unit for mining operations, the predictive uncertainty related to this parameter is significant. The calibrated value for this parameter is also at the lower bound (0.001m/d) of expected range (0.001 to 0.5 m/d).

Scenario 3

For this scenario the hydraulic conductivity of the Styx interburden and overburden were increased concurrently. These units show the smallest sensitivities, and therefore they potentially have a high combined predictive uncertainty.

Scenario 4

For this scenario the hydraulic conductivity of all the Styx coal measures were increased. This scenario is more likely to rapidly break calibration as the underburden conductivity is highly sensitive. However, this analysis aims to identify which condition would be required within the whole coal measures sequence to trigger significant drawdown.

Scenario 5

For this scenario the hydraulic conductivity within the alluvium was increased. The alluvium hydraulic conductivity is the second most sensitive parameter. Increasing the alluvium hydraulic conductivity will likely rapidly impact the calibration and the confidence in its value.

Scenario 6

For this scenario the hydraulic conductivity of the basement was increased. The basement hydraulic conductivity is a sensitive parameter and therefore well constrained by the field observations during calibration. Moreover, the basement unit is not mined, and it is not expected to have a large associated predictive uncertainty. However, this scenario will evaluate whether an insufficient characterisation of the basement unit poses a risk to the model objectives;

Scenario 7

For this scenario all the model parameters described for Scenarios 1 to 6 were increased. This scenario assesses whether higher hydraulic conductivities than calibrated value for all units have the potential to trigger significant impact.

For each scenario, the range of hydraulic conductivities extends from the best calibrated value to at least two orders of magnitude higher. The assessed range covers and exceeds the anticipated range of values for each parameter. For each scenario, between 12 to 20 runs were processed with progressively rising values in order to identify where impact to predicted drawdowns becomes noticeable and to what extent predictions change.

Storativity uncertainty was not assessed in this analysis. Calibrated storativity is at the lowest bound of the pre-calibration range and lowest value of storativity are conservative in terms of drawdown. It makes little sense to decrease this parameter further to assess a potential breaking point. Moreover, storativity is not a particularly sensitive parameters concerning predictive heads as illustrated in the sensitivity analysis (see section [3.7.1\)](#page-89-0).

[Table](#page-113-0) 3-14 summarises the parameter ranges that were explored.

Scenario	Alluvium	Overburden K [m/d]	Interburden K [m/d]	Underburden K [m/d]	Basement
1	4.1	$1.7x10^{-3}$ to 1.7 $x10^{-1}$	0.001	0.001	0.00044
2	4.1	$1.7x10^{-3}$	0.001 to 2	0.001	0.00044
3	4.1	$1.7x10^{-3}$ to 1.7 $x10^{-1}$	0.005 to 2	0.001	0.00044
4	4.1	$1.7x10^{-3}$ to 1.7 $x10^{-1}$	0.001 to 2	0.001 to 0.1	0.00044
5	4.1 to 150	$1.7x10^{-3}$	0.001	0.001	0.00044
6	4.1	$1.7x10^{-3}$	0.001	0.001	0.0004 to 0.044

Table 3-14 Range of hydraulic conductivity values explored

3.7.3.2 Results

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Drawdown at the simulated coast observation point for all scenarios is illustrated on [Figure](#page-116-0) 3-74, and drawdown at the simulated creeks confluence observation point is illustrated on [Figure](#page-117-0) 3-75.

Near the coast³, the various scenarios indicate that:

- The drawdown is negligible $\left($ <0.05m) regardless of the value of hydraulic conductivity in the alluvium (scenario 5), overburden (scenario 1) and basement (scenario 6).
- A drawdown of more than 0.5m is triggered when the hydraulic conductivity of the interburden exceeds 0.5m/d. This is true whether only the interburden hydraulic conductivity is increased (scenario 2) or when it is in conjunction with other units (scenarios 3, 4 and 7).
- With a uniform hydraulic conductivity of $2m/d$ for the interburden, a drawdown maximum of 2 m would be observed near the coast. This is true regardless of the hydraulic conductivity of any of the other units.

At the confluence of Deep and Tooloombah Creek, the various scenarios indicate that:

- The drawdown is small ≤ 0.2 m) regardless of the value of hydraulic conductivity in the Alluvium (scenario 5), overburden (scenario 1) and basement (scenario 6).
- A drawdown of more than 5 m is triggered when the hydraulic conductivity of the interburden exceeds 0.5 m/d. This is true whether only the interburden hydraulic conductivity is increased (scenario 2) or when it is in conjunction with other units (scenarios 3, 4 and 7).
- With a uniform hydraulic conductivity of 2 m/d in the interburden, a maximum drawdown of about 25 m would be observed. This is true regardless of the hydraulic conductivity in any of the other units.

The following remarks can be derived from the 'breaking-point' assessment:

- Misestimation of the hydraulic conductivity within the alluvium, overburden, underburden and basement have only limited predictive consequence regarding the extent of the cone of depression in the modeled potentiometric surface.
- The extent of downstream drawdown within the Styx Basin is mainly controlled by the hydraulic conductivity of the Styx interburden unit. Most of the predictive uncertainty in terms of the extent of the cone of depression relies on the characterisation of the interburden.
- The pre-calibration range for the interburden hydraulic conductivity extends from 0.0005 to 0.5 m/d. At 0.5 m/d a drawdown of about 0.5 m is predicted near the coast and about 5 m at the creeks' confluence. To evaluate whether a uniform hydraulic conductivity of 0.5m/d or higher could reasonably define the regional interburden hydraulic property, for all the scenarios runs, calibration statistics have been calculated for observations located in the interburden unit (see target location on [Figure 3-10](#page-33-0)). As illustrated on [Figure](#page-117-1) 3-76, the best

³ The coast is represented in the model by a constant head BC. To prevent the constant head BC to fully control the head of the observation point, it was located 300m further south

sRMS within the interburden is 5.3%. for the low value of hydraulic conductivity $\left($ <0.01 m/d for scenario 2).

- For values higher than 0.01 m/d the calibration deteriorates progressively and the sRMS is more 20 for any value of hydraulic conductivity above 0.2 m/d for all scenarios, indicating a regional hydraulic conductivity of more than 0.2 m/d is unlikely to be representative.
- A representative regional hydraulic conductivity value of 0.01 m/d or less is more likely to be representative4. For this range of interburden hydraulic conductivity, the cone of drawdown remains within the vicinity of the mine. At the confluence of the two creeks, the drawdown remains below 0.5 m and negligible near the coast (<0.05 m).

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⁴ Note. These estimations are relative to the calibrated regional recharge (se[e Table](#page-34-0) 3-5). For a different value of recharge the values would need to be adjusted accordingly. However, the uncertainty on those values mirroring recharge uncertainty is less than an order of magnitude.

Figure 3-74 Predicted drawdown near the coast

Figure 3-75 Predicted drawdown at the Tooloombah and Deep Creek confluence

Figure 3-76 Interburden sRMS observations versus interburden hydraulic conductivity for all scenarios

The predictive consequence for a range of hydraulic conductivities spanning beyond the precalibration estimated range was explored. The range explored during the uncertainty analysis is illustrated on Figure 3-69.

The hydraulic conditions required for a significant impact are not supported by field water level observations. The conclusions for each HSU are as follow:

Alluvium

The calibrated hydraulic conductivity value is 4.1 m/d . This value is within the pre-calibration expected range. Moreover, it is demonstrated that higher values of hydraulic conductivity for the alluvial aquifer, while being less reliable as they impact calibration do not trigger a significantly wider depression cone, which is probably in relation to the limited depth of the alluvium aquifer.

Overburden

The calibrated hydraulic conductivity of the overburden (0.02 m/d) is in the lower end of the pre-calibration expected range $(0.0005 \text{ to } 0.5 \text{ m/d})$. The e overburden is absent on the western side of the mine pits and therefore is only mined on the eastern side of the mine pits where it overlies the resource bearing interburden. As it is only marginally mined, it has only very limited control on the extension of the drawdown cone as illustrated at both observation points [\(Figure](#page-116-0) 3-74 and [Figure](#page-117-0) 3-75). The influence of the overburden on the predictive uncertainty in terms of the extent of the cone of depression is small and even overly conservative values don't trigger significant impact.

Interburden

The interburden hydraulic conductivity value is the most critical for the assessment. The calibrated value (0.003) is at the lower end of the expected pre-calibration range (0.0005 to 0.5 m/d). At the upper end of the range, predicted drawdown related to mining would extend to the coast. However, such values, even though locally acceptable, are unlikely to be regionally representative of the unit as they are not compatible with the regional gradient within the unit as determined by observed head's calibration. A regional value of 0.01 m/d or less is compatible with the calibration to observed heads and hence more likely.

Underburden and Basement

The underburden and basement hydraulic conductivity are highly sensitive parameters and therefore well constrained by the calibration. There is good confidence in their estimation (respectively 0.004 and 0.0004 m/d for the calibrated recharge). Moreover, the predictive uncertainty related to these units is low mainly as they are located below the targeted mining units.

Vertical anisotropy

The modelling has assumed no vertical anisotropy for the basement and the three Styx coal measures units. This assumption is conservative as demonstrate in chapter [0](#page-90-0) (see [Figure](#page-104-0) 3-65).

Figure 3-77 Hydraulic conductivity ranges (pre-model calibration, calibrated values and uncertainty analysis)

The uncertainty analysis has revealed the hydraulic conductivity of the interburden is the most critical modelled hydraulic property for the impact assessment. It carries most of the predictive uncertainty in terms of the extent of the predicted drawdown. To maintain the calibration of the interburden unit, a representative regional value ought to be lower than 0.01 m/d. With a hydraulic conductivity lower than 0.01m/d less than 0.5 m of drawdown is predicted to reach the confluence of Deep and Tooloombah Creeks. The calibrated regional value of hydraulic conductivity is within the range of the field program (see Chapter 10 of the SEIS, Section 10.5.6.3) which reports of hydraulic conductivities within the Styx Coal Measures spanning from 0.001m/d to 0.3m/d. These values have mainly been obtained by slug tests and therefore have only a local scope, while the numerical model reflects the regional behaviour. Field values indicate a large local variability of hydraulic conductivity within the Styx Basin.

Misestimation of hydraulic conductivity in the other units (alluvium, overburden, underburden and basement) have only very limited predictive consequences in terms of the extent of the depression cone.

3.8 Numerical model conclusions

The numerical model assessment consisted of a calibrated model and an additional 190 models build to assess the predictive uncertainty related to the mining operations;

The following provides summary conclusions for groundwater modelling undertaken for the Project:

1. During mining, the maximum predicted drawdown exceeds 100 m, but this is restricted to ML 80187 immediately surrounding the pits. The 1 m drawdown contour intercepts the midportion of Tooloombah Creek and Deep Creek and the 0.1 m drawdown contour (assumed to represent the zone of drawdown influence) extends to a maximum of approximately 5.5 km northwest of the mine at around year 10 post-mine, and does not intercept Styx River.

- 2. Drawdown of the water table within the Tooloombah and Deep Creek catchments results in dewatering, to some extent, of the alluvial aquifers that likely support the mid- to lower reaches of the two creeks and associated riparian zones.
- 3. The model predictions and predictive uncertainty results also support the conceptualisation that the combined Tooloombah Creek and Deep Creek catchments, within which the Project is located, is essentially a closed groundwater catchment.
- 4. The lack of drawdown in the lower reaches of the tributary catchments (Tooloombah and Deep Creeks) and downstream of the confluence of these creeks indicates the potential for seawater intrusion in response to the Project is negligible.
- 5. The zone of mine-related drawdown influence is predicted to align northwest to southeast, and does not interfere with the tidal reach of Styx River. This drawdown persists for up to 50 years post-mining but, because the mine pits are progressively backfilled, the groundwater system is predicted to fully recover sometime after 50 years (but before 100 years).
- 6. The groundwater model calibration is most sensitive to the hydraulic conductivity of the Styx Coal Measures underburden, the hydraulic conductivity of the alluvium, and recharge rates.
- 7. The predictive uncertainty in term of drawdown extent is mainly controlled by the hydraulic conductivity within the interburden. For this reason, the characterisation of this unit is the most critical to further reduce predictive uncertainty. The uncertainty analysis showed that the calibration of interburden observations can be maintained with a representative regional value lower than 0.01 m/d.
- 8. Predictive uncertainty analysis indicates, in a most severe possible scenario, significant drawdown associated with mine water affecting activities will not extend to areas where there is a potential for exposure of ASS to occur, or along the tidal reach of Styx further. Consequently, any threat to marine and aquatic ecosystems associated with ASS is considered negligible.
- 9. The uncertainty analysis indicates the calibrated model, and the predictions presented in this report, are representative and consistent with the conceptual hydrogeological model, which is presented in SEIS Chapter 10 - Groundwater.
- 10. The zone of influence from mine dewatering activities is predicted to not reverse hydraulic gradients along Styx River, i.e. it remains a dominantly groundwater discharge zone during and following mining. North of ML 80187, continued groundwater discharge to Tooloombah and Deep Creeks is also predicted.

3.9 Model limitations

The geometry and properties of natural groundwater systems can never be fully characterised, and the past and potential future hydrogeological processes can only be inferred from a limited number of uncertain measurements. Therefore, simplifications are necessary and uncertainty is inherent in groundwater modelling.

The groundwater model for the Project has been constructed based on the available geological data (borehole logs, geological maps, CQC geological model) and calibrated to available groundwater heads using hydraulic property values that are consistent with those derived from on-site hydraulic testing and literature review.

Each HSU is represented by one or more layers that have a homogenous and isotropic distribution of hydraulic properties that aim to represent the behaviour of regional aquifers. Local variability of hydraulic properties is not represented in the model. However, the bulk representation of the hydraulic properties is appropriately simulated, given the purpose of the model is to assess regional effects of mine water affecting activities, particularly mine dewatering and depressurisation.

The uncertainty analysis has considered a wide range of possible hydraulic properties for the Project area groundwater system within the anticipated range of possible values. The analysis explored various combinations of properties that can maintain an adequate model calibration. Nevertheless, in the process of generating calibrated sets of parameters, some parameters extend beyond bounds that are considered reasonable based on observations made of the physical system. This represents a conservative approach that has the tendency to extend the range of possible predictive outcomes.

Mining operations are represented with a yearly time step, which is not an accurate representation of actual operations, i.e. mine pits and other facilities evolve over much shorter timeframes. Simulating mining as a yearly progression is conservative area as the progressive nature of pit development, backfilling and dewatering is not considered.

4 Styx River in-stream pools water balance

4.1 Background and objectives

Permanent pools have been observed along Tooloombah Creek and Deep Creek. The pool levels appear to remain relatively stable across the whole dry season (generally from April to July). Groundwater level observations in the alluvium aquifer indicate an upward hydraulic gradient (from aquifer to pools) across the wet and dry seasons, however the gradients are not strong gradients (i.e. there is less than a metre difference in heads).

The primary objective of the mass balance model is to understand the mechanisms that sustain the permanent and semi-permanent pools in the Tooloombah Creek and Deep Creek catchments.

The pool at To2 (see Figure 10-6 of SEIS Chapter 10 – Groundwater) pool was selected for this assessment as it is the main largest permanent pool in the closest vicinity of the mining operations with sufficient data available to apply a mass balance model.

4.2 Conceptualisation

[Figure](#page-122-0) 4-1 illustrates the conceptual understanding of permanent pool hydrology. The water level in a pool is a balance between the pool inflows (incident rainfall, rainfall runoff, groundwater inflow, transfer along alluvial sediments) and outflows (evaporation, leakage to groundwater, transfer along alluvial sediments).

Figure 4-1 Pool conceptualisation

The mass balance can take the form of equation 1. $\Delta_{Storage} = I_{GW} + I_P + I_R + I_S - O_E - O_S - O_{GW}$ (1)

During a dry period (defined as the period with no stream flow) with a stable pool water levels, the terms I_R , I_H , O_H , O_{GW} and $\mathbb{Z}_{\text{Storage}}$ become negligible, and the mass balance for a permanent or semipermanent pool becomes:

$$
\Delta_{Storage} = I_{GW} + I_P + O_E = 0\tag{2}
$$

or

1

$$
I_{GW} = O_E - I_P \tag{3}
$$

For a dry period, the groundwater contribution to the pool (*IGW*) is equal to the amount removed by evaporation (O_E) less incident rainfall over the pool.

The groundwater contribution component is made up of water from a number of possible sources. Understanding and quantifying the various contributions is key to predicting the potential impact of mine water affecting activities on the pool water body as not all components will be equally affected.

The possible sources of water for the alluvial aquifer include:

- Rainfall diffuse recharge Infiltration of rainfall water directly to the alluvial aquifer.
- Bank storage The recharge of the near stream bank by streamflow during a flood event.
- Aquifer interactions The exchange of water between adjacent and/or deeper aquifers and the alluvium. The contribution is positive if the head in the aquifer(s) is higher than in the alluvium aquifer, and negative otherwise.
- The creek hyporheic zone The stream bed is recharged during a stream flow event. The upstream positive contribution to a pool might be matched by a similar negative contribution downstream of the pool if the structure and profile of the alluvium remains unchanged and evaporation is ignored.

Quantifying the respective contribution of all these parameters would be a complex task as the processes change both temporally and spatially [\(Table](#page-124-0) 4-1) and because only limited information can be made available to characterise them. The assessment could be made by exploring various lines of evidence including geochemical evidence (identify the chemical signature of each source

 5 I_{GW} is made of groundwater from various contribution including bank storage release (I_B), contribution from the creek hyporheic zone (Iss), from deeper aquifer and from the diffuse rainfall recharge over the alluvium and generating potentially localised perched conditions.

and modelling the potential mixing) and physical evidence (based on respective water level observation; see [Table](#page-125-0) 4-2 for detail).

A detailed analysis of those contributions would allow to understand which portion of the groundwater inflow will be affected by the mining operation. A first approximation and conservative approach is to assume that all the groundwater contribution will be affected by the mining operation.

As illustrated in [Table](#page-124-0) 4-1, the key water source for the alluvial aquifer that may be affected by the proposed mining operation is "aquifer interaction".

Component	Dynamic	Spatial distribution	Affected by mining operations
Bank storage return	The recharge of the alluvial aquifer - episodic and brief (possibly less than 1 month).	Localised in the vicinity of the pool. The extension is controlled by the area flooded and the water level in the creeks during flood events.	Limited as diversions of watercourses are not proposed. However, the bank return could be altered if the vertical gradient between the alluvium and underlaying unit is inverted during mining operations
Diffuse rainfall recharge	Seasonally variable. Controlled by soil and unsaturated zone water content, rainfall patterns and evapotranspiration.	Spread uniformly over the extent of alluvium.	Limited to the extent of the mine activities. Watercourses are unaffected.
Sub surface flow	The recharge of the alluvial aquifer is episodic and brief (possibly less than 1 month).	Localised upstream and downstream, including the pool (small transversal section).	Limited as watercourse diversions are not proposed. However, sub surface flow could be altered if the vertical gradient between the alluvium and underlaying unit is inverted during mining operations
Aquifer interactions	Relatively stable. Controlled by the head differences between heads in the adjacent and/or deeper aquifer(s) and the alluvial aquifer.	Spread variably/unevenly over the extent of the alluvium. The general gradient would support discharge of water from deeper aquifers (and the alluvium) near to watercourses and vice versa away from the watercourses.	The drawdown caused by mine dewatering will alter these dynamics during mining and for some time after the completion of mining.

Table 4-1 Characterisation of the alluvium aquifer

Component	Hydrochemistry	Physical constraint	Other assessments approaches
Bank storage return	Young water (weeks) with little mineralization. Hydrochemical signature likely to be similar to rainfall.	Limited extent restrained by the flood zone, with bank storage estimates based on physical dimensions.	- Bank storage volume assessment - Analytical model - 2D cross section numerical model
Diffuse rainfall recharge	Young water (years). Chloride and Carbonate likely to be dominant anions, and sodium the dominant cation.	Evapotranspiration demand, soil type(s), rainfall patterns and intensity.	- Bucket type model - Unsaturated / saturated model - Analytical model - 2D or 3D numerical models
Sub surface flow	Young water (weeks) with little mineralization. Hydrochemical signature likely to be similar to rainfall.	Pre-stream flow moisture content of sediments, homogeneity of hyporheic zone (connectivity, hydraulic conductivity, cross-sectional area).	- Analytical solution (Darcy) - Numerical solution
Aquifer interactions	Old water (>10 years). Hydrochemical signature likely to be similar to deeper aquifer(s). Chloride likely to be dominant anion, and sodium the dominant cation.	Hydraulic head gradients (direction and scale), vertical hydraulic conductivity in sediments and Styx Coal Measures	- Analytical solution (Darcy) - 3D numerical model (check from the existing model)

Table 4-2 Approach to identify pool groundwater contribution

4.3 Pool water balance calculations

The pool at "To2" is on average approximately one meter deep and has a total surface area of around 4,060 m2.

For the period from April to July 2018, minor rainfall occurred in the Project area (total of 60 mm for the 4 months, for an average of 0.5 mm/d) and the creeks didn't experience any stream flow. Over the same period the average rate of evapotranspiration is estimated at 4.1 mm/d. The pool water level has remained stable during the period. By applying equation (2) over this dry period, the groundwater contribution is estimated about 3.6mm/d. Assuming that all groundwater contribution will be affected by mining operations, 3.6mm/d would need to be supplied for each affected pool to sustain their level during the dry seasons.

Over the To2 pool area, rainfall is estimated to have contributed approximately 244 m³ (0.5 mm/d over 4,060 m² for 4 months), and ET is estimated to remove around 2000 m³ (4.1mm/d over 4,060m² for 4 months). Based on equation (2), groundwater is estimated to have contributed 1.756 m³ (14.6 m³/d, or around 3.6 mm/d). For a longer dry season (i.e. time between creek flow events) of six months long, at 3.6mm/d it is conservatively extrapolated that groundwater might contribute 2,635 m³ to the permanent pool.

The numerical model (Section 3) estimated that a well in the Styx Coal Measures could sustain an average pumping of 0.7L/s (60m3/d). At this rate, the well could support 17,000m² of pool during the dry season. Such a well could be enough to support To2 water level during the dry season.

The water balance model indicates the amount of water required to sustain in-stream pools during the dry season is around 3.6 mm/d.

5 Targeted GDE investigations

5.1 Introduction

Targeted investigations were undertaken in August 2018 to better understand vegetation water use at three sites where groundwater use by vegetation was considered likely (see [Figure](#page-127-0) 5-1).

The Wetland 1 Assessment Area is listed under the Vegetation Management Act as having High Ecological Value and is also listed as a Wetland Protection Area. It is identified in the BoM GDE Atlas as an artificial/highly modified wetland reliant on surface expression of groundwater. It consists of a clay plan, inhabited predominantly by broad leaf tea tree (*Melaleuca viridiflora*). A single red gum (*Eucalyptus tereticornis*) is located in the centre of the wetland. Soils are heavy clays to a depth of 1.5 m.

The Wetland 2 Assessment Area is also listed under the Vegetation Management Act as having High Ecological Value and is identified in the BoM GDE Atlas as a coastal/sub-coastal floodplain swamp reliant on surface expression of groundwater. It comprises an internally draining swamp that inhabited by sedges and fringed by larger red gums (*E. tereticornis*).

The Alluvial Vine Thicket Assessment Area is located on the southern alluvial terraces of Tooloombah Creek and consists of a community of low canopy (7–10 m) trees comprising a variety of species with occasional emergent red gums (*E. tereticornis*). A varied understory with abundant vines is present. Soils are relatively sandy.

5.2 Methods

5.2.1 Theory

Several tests can be used to evaluate the dependence of vegetation on groundwater (Eamus 2009). Each test is subject to limitations, but a combination can be used to provide a robust assessment in which multiple lines of evidence are examined. The following tests have been selected for this investigation:

Stable isotopes of water: Stable isotopes are those that do not radioactively decay over time. The stable isotopes of water are those derived from hydrogen (deuterium; 2H) and oxygen (18 0). The ratio of ²H to ¹H and the ratio of 18 O to 16 O varies for different bodies of water due to isotopic fractionation caused by transport processes and phase transitions through the atmosphere, lithosphere and biosphere (Barnes and Alison 1988). Since the fractionation processes are likely to be different for groundwater, surface water and soil water, different sources of plant water will often, but not always, have different isotopic compositions that will be reflected in the plant xylem water. Knowing the isotopic composition of these sources and plant water can assist in conceptualising plant water uptake.

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Soil-plant water relations: Plants require water for photosynthesis. Transpiration is the process of plant water use in which plants take up water through their roots, passively transport this water through the xylem, and regulate the diffusive evaporation of this water through the stomata in their leaves. The rate of transpiration is controlled by solar radiation, the evaporative demand of the atmosphere (which is influenced by temperature, humidity, wind speed and incident sunlight), soil water supply, and stomatal regulation. For plants to transpire they must maintain a leaf water potential (LWP) that is more negative than the soil water potential (SWP). A dry soil will have a highly negative SWP and a plant must be able to regulate their stomatal conductance and lower their LWP to be more negative than the SWP to extract water. By contrast, a moist soil or one that is maintained in moist state by shallow groundwater will not require plants to lower their LWP excessively to extract water. Thus, measuring LWP and SWP concurrently provides an indication of where in the soil profile plants are drawing their water from and it also provides an indication as to whether plants have access to groundwater (at the water table, SWP approaches zero). Measurements of LWP are usually made pre-dawn when the difference between LWP and SWP is likely to be smallest because nocturnal transpiration is minimal (Ritchie and Hinckley 1975).

5.2.2 Drilling, sampling and analysis

Six boreholes were drilled as shown in [Figure](#page-127-0) 5-1. BH1, BH2 and BH3 were drilled at Wetland 1. BH4 and BH5 were drilled at Wetland 2. BH6 was drilled at the Alluvial Vine Thicket site. Each of the boreholes were drilled very near to the trees that were sampled and analysed.

Drilling was undertaken using a combination of direct push, and RC drilling methods. Direct push tubes were taken until refusal, with extended drilling undertaken using an RC rig. Care was taken to avoid the use of RC drilling and use of drilling fluids where possible—which compromise soil moisture and soil water isotope analysis—but in a few cases this could not be avoided, and drilling continued with the use of applied water to the base of the borehole. It is also noted that the use of air likely influenced the moisture content of the samples returned. Drilling logs are provided in Attachment 1 and a summary of the drilling undertaken is outlined i[n Table](#page-128-0) 5-1.

Samples were taken in metre increments or at a finer scale. For isotope analysis, samples were immediately sealed in an air-tight sample bag, kept on ice and transported to a freezer before dispatch to the ANU Stable Isotope Laboratory where they were analysed. Soil moisture potential was measured in the field on sub-samples using a WP4C dew-point potentiometer from push tube or from RC cuttings.

5.2.3 Vegetation sampling and analysis

Pre-dawn LWP measurements were made using a pressure chamber device. Measurements were taken on leaves collected from several trees in close proximity to the boreholes at each site. Stem samples were also taken from the outer branches of trees for isotope analysis. As per sampling protocols, they were cut into 5 cm lengths, stripped of bark, stored in leak-proof containers, kept on ice and transported to a freezer for dispatch to the ANU Stable Isotope Laboratory where they were analysed.

5.2.4 Well installation and groundwater sampling

Groundwater wells were installed in September 2018 to monitor the water table at Wetland 1 (WMP25) and Wetland 2 (WMP27). These wells will form part of the monitoring network and can be sampled for isotope analysis if necessary.

5.3 Results

5.3.1 Wetland 1

Drilling logs for the boreholes completed at Wetland 1 (BH1, BH2 and BH3) are shown in Attachment 1. The lithology consists of heavy clay at the surface associated with the clay plan, which is underlain by calcrete bands and layers of sandstone and clay. A moist zone was encountered from 8 mBGL (alluvials and calcrete) and the water table at 10.2 mBGL.

Soil and leaf water potentials at Wetland 1 are presented on [Figure](#page-130-0) 5-2. Much of the soil profile is very dry and well below the agronomic wilting point of -1.5 MPa, noting that native tree species can often tolerate soil moisture potentials well below this level. Root water uptake is indicated where the leaf water potentials are equivalent to or less than the soil water potential. For two of the trees measured, Tree 1 (*M. viridiflora*) and Tree 4a(*E. tereticornis*) root water uptake was indicated from depths of 0.5 to 1.0 m. For the remaining trees, several *M. viridiflora* trees and one *E. tereticornis* specimen, root water uptake was indicated at 8 m where the soil water reservoir appears to be more moist than higher up, possibly linked to the capillary fringe above the water table or to a 'perching' layer in the soil profile at around 8 m depth. The soil samples below the water table do not reflect saturated conditions and may have been influenced by the drilling method which required compressed air to retrieve the samples.

Figure 5-2 Soil and leaf water potentials with depth, Wetland 1

The stable isotopes of water in soil water and vegetation are plotted with respect to depth in [Figure](#page-131-0) 5-3. The shallowest soil sample at 0.1 m is enriched in heavy isotopes due to the fractionation that occurs during evaporation whereby the lighter, standard water molecules (composed of 1H and ¹⁶O) are preferentially converted from the liquid to the vapour phase, compared to heavier water molecules composed on 2H and 18O. The isotopic enrichment is mostly confined to the upper few metres of the soil profile.

Groundwater is yet to be sampled directly for isotopic characterisation but the slight enrichment in the deeper samples, below 8m, may reflect the influence of a groundwater signature.

[Figure](#page-132-0) 5-4 presents a scatter plot of $δ¹⁸O$ against $δ²H$. The Global Meteoric Water Line (GMWL) is presented as an approximate reference for rainfall, noting there may be local deviations from this relationship due to local meteorological processes. The relative enrichment of the 18O molecule reflects an evaporation trend⁶ which is interpreted o[n Figure](#page-132-0) 5-4. The vegetation at Wetland 1 has xylem water that plots along this trendline, indicating the xylem water has been sourced primarily from the upper 2 m of the soil profile.

Figure 5-3 Stable isotopes of water in vegetation samples and in soil water with depth, Wetland 1

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 $⁶$ Because oxygen has a higher atomic weight than hydrogen, the $¹H₂¹⁸O$ molecule is less likely to change</sup></sup> from the liquid to the vapour phase than the ¹H²H¹⁶O molecule. For this reason, the relationship between δ^{18} O and δ^2 H will deviate from the GMWL during evaporation (Sprenger et al. 2016).

Figure 5-4 Stable isotopes of water in soil and vegetation samples, Wetland 1 (GMWL: Global Meteoric Water Line)

Together, the water potential measurements and the isotope data indicate vegetation at Wetland 1 is sourcing most of its water from the near surface, and maintenance of the surface hydrological regime (run-off and inundation) of the wetland is critically important for maintaining the wetland's environmental water requirements. However, the trees may access deep soil water that is maintained by groundwater to support transpiration requirements during sustained dry periods when the soil water reservoir is otherwise depleted.

5.3.2 Wetland 2

Drilling logs for the boreholes and well completed at Wetland 2 (BH4, BH5 and WMP27) are shown in Attachment 1. The lithology consists of clay at the surface underlain by layers of sand and clay, with sandstone from 15 mBGL. The water table is at about 20 mBGL.

Soil and leaf water potentials at Wetland 2 are presented in [Figure](#page-133-0) 5-5 . Much of the soil profile is very dry and well below the agronomic wilting point of -1.5 MPa, noting that native tree species can often tolerate soil moisture potentials well below this level. Root water uptake is indicated where the leaf water potentials are equivalent to or less than the soil water potential and the results point to root uptake from depths of 2 to 4 m at the time of sampling. There is no indication of groundwater use at this site.

Figure 5-5 Soil and leaf water potentials with depth, Wetland 2

The stable isotopes of water in soil water and vegetation are plotted with respect to depth in [Figure](#page-134-0) 5-6. Samples very near to the surface (e.g. at 0.1 m depth) were not analysed at Wetland 2 so an evaporative zone is not indicated. There is also a clear distinction between the samples taken at BH4 and BH5. This could be due to spatial variability, differences in sampling methods (push-tube vs RC drilling) or a combination of both.

[Figure](#page-134-1) 5-7 presents a scatter plot of $δ¹⁸O$ against $δ²H$. The vegetation at Wetland 2 has Xylem water which is enriched in ¹⁸O relative to the GMWL and is consistent with the majority of soil water samples at BH5. This is indicative of root water uptake throughout the soil profile. No groundwater use is indicated at Wetland 2 from either the water potential measurements or the isotope data.

No groundwater use is indicated at Wetland 2 from either the water potential measurements or the isotope data. The assessment presented indicates that maintenance of the surface hydrological regime (run-off and wetland inundation) as being critically important for maintenance of Wetland 2 environmental water requirements.

Figure 5-6 Stable isotopes of water in vegetation samples and in soil water with depth, Wetland 2

Figure 5-7 Stable isotopes of water in soil and vegetation samples, Wetland 2 (GMWL: Global Meteoric Water Line)

5.3.3 Vine thicket

The drilling log for the borehole completed at the Vine Thicket (BH6) is shown in Attachment 1. The lithology consists of mostly sandy sediments to a depth of 8.5 m, where weathered sandstone is encountered. The water table was not encountered during drilling, thus it is at least 10 mBGL.

Soil and leaf water potentials at the Vine Thicket are shown in [Figure](#page-135-0) 5-8 . Much of the soil profile is very dry and well below the agronomic wilting point of -1.5 MPa, noting that native tree species can often tolerate soil moisture potentials well below this level. Root water uptake is indicated where the leaf water potentials are equivalent to or less than the soil water potential and the results point to root uptake from depths of 2 to 3 m at the time of sampling. There is no indication of groundwater use.

Figure 5-8 Soil and leaf water potentials with depth - Vine Thicket vegetation

The stable isotopes of water in soil water and vegetation are plotted with respect to depth in [Figure](#page-136-0) 5-9. The shallowest soil sample at 0.2 m is enriched in heavy isotopes due to the fractionation that occurs during evaporation whereby the lighter, standard water molecules (composed of 1H and ¹⁶O) are preferentially converted from the liquid to the vapour phase, compared to heavier water molecules composed on 2H and 18O. The isotopic enrichment is mostly confined to this near-surface zone.

Figure 5-9 Stable isotopes of water in vegetation samples and in soil water with depth at the Vine Thicket

[Figure](#page-137-0) 5-10 presents a scatter plot of δ^{18} O against δ^2 H. The relative enrichment of the ¹⁸O molecule reflects an evaporation trend which is interpreted o[n Figure](#page-137-0) 5-10. The vegetation at vine thicket has xylem water which plots along this trendline, indicating the xylem water has been sourced primarily from the top 0.5 m of the soil profile.

There is no indication of groundwater use by the Vine Thicket vegetation. The assessment presented indicates that maintenance of the surface hydrological regime (stream flows and run-off) as being critically important for maintenance of Vine Thicket environmental water requirements.

Figure 5-10 Stable isotopes of water in soil and vegetation samples, Vine Thicket (GMWL: Global Meteoric Water Line)

6 Connectivity assessment using isotope analysis

6.1 Overview

This preliminary isotope study has been undertaken to provide an initial indication of water sources supporting the watercourse pools, which are hypothesised to be supported to some extent by groundwater discharge (i.e. Type 2 GDEs, see SEIS Chapter 10 - Groundwater). This study forms part of the planned additional scope of works required to assist in improving the understanding of the water requirements of the GDEs and potential impacts arising from mining operations which will in turn enable appropriate management objectives and approaches to be developed to manage these GDEs during and post-mining.

To better understand the relationship between the surface water and groundwater, the stable isotopes of water (δ^2 H and δ^{18} O) and radon isotope (222 Rn) were analysed from water samples collected from the site.

6.2 Environmental (stable) isotopes

Craig (1961) observed that when stable isotopes of water (δ^2 H and δ^{18} O) have not undergone evaporation, they would have a linear relationship which can be represented by:

 δ^2 H = 8 δ^{18} O + 10

This equation is referred to as the "Global Meteoric Water Line" (GMWL) and was developed based on precipitation data from across the globe. A "Local Meteoric Water Line" (LMWL) is usually developed from precipitation data collected from either a single location or a set of locations within a "localised" area of interest (USGS, 2018), noting that there are limited data available in the project area to construct this trend, thus there is some uncertainty in the LMWL derived.

The stable isotopes of water can be used to discriminate between different sources of water. The method relies on the distinct isotopic compositions which can arise as a result of isotopic fractionation caused mainly by transportation processes (i.e. mixing) and phase transitions (i.e. evaporation) through the atmosphere, lithosphere and biosphere (Barnes and Allison, 1988).

Six grab water samples were collected from Tooloombah Creek in-stream pools (two sampling points each from three pools within the creek) and another three from Deep Creek in-stream pools (a sampling point from three pools within the creek) between 16th and 18th July 2018. Groundwater samples were collected from six monitoring wells that are close to the surface water sampling points, using a low-flow groundwater sampling pump.

Water samples were analysed by Environmental Isotopes (contracted via Australian Laboratory Services, ALS). The hydrogen and oxygen isotope ratios were measured using a Wavelength Scanned-Cavity Ring-down Spectrometer (Picarro L2120) based on Munksgaard *et al*. (2011). Lake Eacham and Cairns Tap water were used as reference waters to develop localised water standards.

Laboratory results are presented in [Table](#page-139-0) 6-1 while the results and water standards along with the relevant MWLs are plotted in [Figure 6-1.](#page-139-1)

Location	Sample type	δ2H (‰, VSMOW)	δ180 (‰, VSMOW)
WMP02		-23.11	-3.94
WMP04	Groundwater	-27.23	-4.42
WMP04D		-25.12	-4.23
WMP05		-34.07	-5.44
WMP06		-11.77	-1.69
WMP10		-24.06	-4.04
De ₂	Surface water:	2.34	1.28
De3	Deep Creek	-14.4	-1.71
De5		0.37	0.92
T01.1		2.04	0.9
T01.2		0.49	0.65
T02.1	Surface water:	-0.07	0.76
T02.2	Tooloombah Creek	-7.98	-0.61
T03.1		-1.87	0.45
T03.2		1.51	1.07

Table 6-1 Stable isotopes of water measured in groundwater and surface water samples

Figure 6-1 Environmental (stable) isotopes

The Global Meteoric Water Line is based on precipitation data from numerous locations across the globe while the Local Meteoric Water Line was developed from data collected by CSIRO between February and March 2010 in Rockhampton, Queensland (Crosbie et al., 2012).

The laboratory reported Lake Eacham and Cairns Tapwater are reference standards that the samples can be compared with. There is clear distinction between the isotopic values of samples from the surface water and groundwater, indicating different processes have affected the two sample groups. Gonfiantini, (1986) noted that when water undergoes evaporation, the residual isotopes become progressively enriched in heavier isotopes of δ^2 H and δ^{18} O, and the ratio of δ^{18} O to δ^2 H increases. However, when isotopic composition of a water sample plots close to the MWL, it is indicative of its meteoric origin.

The groundwater samples plot on or near the GMWL and LMWL, indicating that groundwater is derived mainly from rainfall recharge and that they underwent little to no evaporation prior to recharge. Surface water samples collected from in-stream pools plot well below the GMWL and LMWL, indicating that they have been affected by evaporation (heavier isotopes, relative to the groundwater samples), which is not unexpected. The data suggest that groundwater could be the source of water sampled from the pools.

6.3 Radioactive isotopes - Radon

Radon-222 (²²²Rn) is a radioactive daughter isotope of Radium-226 and is the longest-lived and most-studied isotope of radon (it has a half-life of 3.82 days). Radon is a gas and will tend to seek a gaseous phase. This means that the natural waters that come into contact with the atmosphere will readily lose radon to the atmosphere, which has very low radon concentrations (USGS, 2018). For this reason, groundwater usually has a much higher concentration of $222Rn$ than surface water.

²²²Rn is often used in groundwater and surface water interaction studies because the surface water will have a very low natural ²²²Rn due to degassing and any elevated concentrations can be used to indicate local discharges of groundwater.

Six grab water samples were collected from the Deep and Tooloombah Creeks (three each) between 16th and 18th July 2018. Groundwater samples were not collected for radon analysis during this preliminary assessment as the current monitoring bores are not in proximity to surface water bodies. Radon was extracted from the groundwater samples into a 20 mL mineral oil scintillant based on the method proposed by Leaney & Herczeg (2006).

Water samples were analysed on the 26th July 2018 at the Australian Nuclear Science and Technology Organisation (ANSTO) laboratory.

[Table](#page-140-0) 6-2 provides a summary of the radon results. Notably, the holding period between the sampling and analysis was between 8 and 10 days, and given $222Rn$ has a 3-day half-life, corrections have been required to account for this (using a correction equation described by Dawood *et al*., 2012).

Table 6-2 ²²²Radon data

The ranges of observed ²²²Rn concentrations in the pools at Tooloombah Creek indicate that there is a likely connection between the creek and the groundwater while the concentrations from Deep Creek indicate low connectivity during the time of sampling (July 2018, which is dry season).

With the understanding that radon, bicarbonate and chloride are higher in groundwater than in surface water and that long residence time would facilitate the loss of radon from water to the atmosphere, O'Grady *et al*. (2007) plotted 222Rn isotopes against chloride and 222Rn against bicarbonate/chloride to determine the amount of groundwater inflow into wetlands. Comparing the ²²²Rn and Cl⁻ present in different locations, the authors noted that sites with:

- **low ²²²Rn and low Cl· concentrations are considered to have short residence times and low** groundwater input;
- **•** high ²²²Rn and medium Cl- concentrations are considered to have short residence times and a reasonable input of groundwater;
- high ²²²Rn and very low Cl- concentrations are considered to be unlikely as low chloride concentration indicates surface water is predominant, which would have negligible radon concentration; and
- high ²²²Rn and high Cl- concentrations are also considered to be unlikely as high concentrations of Cl- would indicate long residence time resulting in high evaporation which would have led to the loss of 222Rn to the atmosphere, exceptional cases include where groundwater is rich in chloride.

Comparing the ²²²Rn and the ratio of bicarbonate and chloride values ($HCO₃$ /Cl·) present in different locations, the authors also noted that sites with:

- **low** ²²²Rn concentration and low ratio of HCO_3 / Cl are considered to have low groundwater input;
- \blacksquare low ²²²Rn concentration and high ratio of HCO₃ / Cl· are considered to have higher groundwater input and long residence;
- **EXECUTE:** high ²²²Rn concentration and high ratio of HCO₃ $/Cl$ are considered to have higher groundwater input and short residence; and
- \blacksquare high ²²²Rn concentration and low ratio of HCO₃ /Cl· are considered to be unlikely as a low ratio of bicarbonate to chloride indicates that groundwater input is negligible, meaning radon should not be present.

[Figure 6-2](#page-142-0) and [Figure 6-3](#page-142-1) show the plots of radon vs. chloride concentrations and radon vs. bicarbonate/chloride ratios, respectively. [Figure 6-2i](#page-142-0)ndicates that groundwater contributes only a limited amount of water to Deep Creek (very low chloride and 222Rn) while Tooloombah Creek receives relatively higher amount of groundwater inflow (higher amounts of chloride and 222Rn). Deep Creek also has longer residence time relative to Tooloombah Creek due to lower 222Rn values. This is further buttressed by [Figure 6-3,](#page-142-1) which indicates that groundwater contributes to some extent in both creeks (medium values for the bicarbonate/chloride ratio, 0.4 – 1.8) at the time of sampling.

Overall, both creeks appear to connected to groundwater to some extent and undergo evaporation.

6.4 Further works

Further works could include collection of samples in the wet and dry season for both environmental and radon isotope analysis from both surface water and groundwater. Shallow in-stream piezometers should be used due to the radon's short half-life (3.82 days). Groundwater samples will also be collected from these piezometers and used for radon analysis to help quantify the groundwater flux. The flux can be calculated using the following equation (Atkinson *et al*., 2015):

I = $(Q*(δ C_r/δx) - w.E. C_r + k.d.w. C_r + λ.d.w. C_r) / (C_i - C_r)$

Where,

I = groundwater inflow $(m^3/m/day)$

 C_i = dissolved ²²²Rn activity of groundwater (Bq/m³)

- C_r = dissolved ²²²Rn activity of surface water (Bq/m³)
- $Q =$ stream discharge, in m³/day
- w = stream width (m)
- $d =$ stream depth (m)
- λ = radioactive decay rate (1/day)
- $E =$ evaporation rate (m/day)

K = reaeration coefficient, in 1/day (range is from 0.5 to 5 (Atkinson et al., 2015))

6.5 Conclusion

The environmental isotopes along with the radon isotopes indicate the creeks are, to some extent, connected to groundwater at the time of sampling. The clear distinction between the environmental isotopes of groundwater and the surface water samples indicate surface water has undergone more evaporation (longer residence time) relative to groundwater. The groundwater samples are less depleted and where they plot on the global and local MWL indicate rainfall is the main source of recharge and that the water underwent little to no evaporation prior to recharge.

 $δ²H$ and $δ¹⁸O$ results from the Deep and Tooloombah Creeks pools shows the isotopes have been enriched as a result of evaporation. It also indicates they have higher retention time (relative to the recharge source of the groundwater).

The radon analysis on Deep and Tooloombah Creeks water samples indicate Deep Creek is less connected to groundwater and has longer residence time relative to the Tooloombah Creek (relatively higher chloride and 222Rn). The presence of bicarbonate and chloride in both creeks (HCO³ -/Cl- ratio ranging between 0.4 and 1.9) indicate groundwater baseflow supports creek pools, albeit not in significant quantities.
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Attachment A - Project water bore logs and soil

core logs

J.

Attachment B - Aquifer test results

Well ID WMP02 **Initial water level** 18.204 m btoc **Well depth** 18.4 m bgl **Screen interval** 12.0 - 18.0 m bgl **Inferred HSU** Alluvium

Well details Analysis details

Inferred HSUs Alluvium **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP04 **Test type** Slug test (Falling head test) **Initial water level** 12.216 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 18.4 m bgl **18.4 m** bgl **H**vorslev (1951) **Screen interval** 12.0 - 18.0 m bgl **Analysis method** AQTESOLV 0.01 m/day 0.02 m/day Hvorslev Bouwer-Rice

Test data

Well ID WMP04D **Test type** Slug test (Falling head test) **Initial water level** 14.0 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 36.5 m bgl **36.5 m bgl HVOTS** 200 m and the set of the HVOTS HVOTS 200 m HVOT **Screen interval** 18.5 - 36.3 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium / Styx Coal Measures - overburden **Hydraulic conductivity (K) Well details**

Analysis details

0.02 m/day Bouwer-Rice

Hvorslev

Well ID WMP06 **Test type** Slug test (Falling Head Test) **Initial water level** 17.767 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 18.4 m bgl **Screen interval** 12.0 - 18.0 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium / Styx Coal Measures - underburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice 0.01 m/day

Well depth 16.0 m bgl **Screen interval** 10.0 - 16.0 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP08 **Test type** Slug test (Falling Head Test) **Initial water level** 12.953 m btoc **Analysis solutions** Bouwer-Rice (1976)

> 0.0005 m/day Bouwer-Rice

Inferred HSUs Alluvium **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP09 **Test type** Slug test (Falling head test) **Initial water level** 12.085 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 15.4 m bgl **15.4 m** bgl **HVOrslev** (1951) **Screen interval** 7.1 - 15.0 m bgl **Analysis method** AQTESOLV 0.1 m/day 0.2 m/day Bouwer-Rice Hvorslev

Test data

Well ID WMP10 **Test type** Slug test (Falling head test) **Initial water level** 8.558 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 18.4 m bgl **Screen interval** 12.0 - 18.0 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - overburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice

0.004 m/day

Test data

 0.0 0.2 0.4 Residual Drawdown (m) 0.6 0.8 1.0 1.2 1.4 1.6 0.01 0.1 $\,1$ 10 100 1000 t/t'

Well ID WMP13 **Test type** Slug test (Falling head test) **Initial water level** 14.319 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 19.7 m bgl **Screen interval** 12.7 - 19.7 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium / Styx Coal Measures - overburden **Hydraulic conductivity (K)**

Well details Analysis details

0.3 m/day

Bouwer-Rice

Initial water level 13.83 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 31.5 m bgl **Screen interval** 25.5 - 31.5 m bgl **Inferred HSUs** Styx Coal Measures - Overburden

Well details Analysis details

Well ID WMP16 **Test type** Slug test (Falling head test)

Well ID WMP16D **Test type** Slug test (Falling head test) **Initial water level** 14.13 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 42 m bgl **Screen interval** 35.7 - 41.7 m bgl **Inferred HSUs** Styx Coal Measures - Interburden **Well details Analysis details**

 $1.2E+3$ $2.4E+3$ $3.6E+3$ $4.8E+3$ $6.0E+3$

 \mathbf{I}

Time (sec)

0.1

 $\overline{1}$

Well ID WMP16D **Test type** Slug test (Rising head test) **Initial water level** 14.13 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 42 m bgl **Screen interval** 35.7 - 41.7 m bgl **Inferred HSUs** Styx Coal Measures - Interburden **Well details Analysis details Analysis details**

Well depth 24 m bgl **Screen interval** 21 - 24 m bgl **Inferred HSUs** Styx Coal Measures - Overburden **Hydraulic Conductivity (H)**

Well details Analysis details

Well ID WMP17D **Test type** Slug test (Falling head test) **Initial water level** 13.13 m btoc **Analysis solutions** Bouwer-Rice (1976)

Late 0.01 m/day

day/۱

Test data

Initial water level 13.13 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 24 m bgl **Screen interval** 21 - 24 m bgl **Inferred HSUs** Styx Coal Measures - Overburden **H Well details Analysis details**

Well ID WMP17D **Test type** Slug test (Rising head test)

Test data 13.10 13.15 13.20 13.25 13.30 13.35 13.40 13.45 13.50 13.55 0 500 1000 1500 2000 Depth to water (m btoc) Time (seconds) ----- Initial level WMP17D slug test WMP17D

Initial water level 14.54 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 23.5 m bgl **Screen interval** 20.5 - 23.5 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - Overburden **Hydraulic conductivity (K)**

Well details Analysis details Analysis details

Well ID WMP18D **Test type** Slug test (Falling head test)

Early 0.7 m/day Mid-late 0.02 m/day Bouwer-Rice

Test data

Initial water level 13.68 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 16.1 m bgl **Screen interval** 13.1 - 16.1 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Weathered Basement **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP19 **Test type** Slug test (Falling head test)

Bouwer-Rice 0.006 m/day

Initial water level 13.63 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 28 m bgl **Screen interval** 24.9 - 27.9 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Weathered Basement **Hydraulic conductivity (K)**

Well details Analysis details Analysis details

Well ID WMP19D **Test type** Slug test (Falling head test)

Early 0.6 m/day Mid-late 0.3 m/day Bouwer-Rice

Test data

Initial water level 11.58 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 20.5 m bgl **Screen interval** 14.5 - 20.5 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - Overburden **Hydraulic conductivity (K)**

Well details Analysis details Analysis details

Well ID WMP20 **Test type** Slug test (Falling head test)

Bouwer-Rice

Early 0.1 m/day Late 0.0004 m/day

Test type Slug test (Rising head test)

Well ID WMP20D **Test type** Slug test (Falling head test) **Initial water level** 12.41 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 30 m bgl **Screen interval** 24 - 30 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - Overburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice

Mid 0.06 m/day Late 0.003 m/day

Test data

Initial water level 12.41 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 30 m bgl **Screen interval** 24 - 30 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - Overburden **Hydraulic conductivity (K) Well details Analysis details**

Well ID WMP20D **Test type** Slug test (Rising head test)

Bouwer-Rice

Early 0.09 m/day Mid 0.02 m/day

Test data

Well ID WMP21D **Test type** Slug test (Falling head test) **Initial water level** 15.07 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 22 m bgl **Screen interval 14 - 20 m bgl Analysis method** AQTESOLV **Inferred HSUs** Alluvium / Styx Coal Measures - overburden **Hydraulic conductivity (K) Well details Analysis details Analysis details**

Bouwer-Rice

0.1 m/day

Well ID WMP22A **Test type** Slug test (Rising head test) **Initial water level** 14.67 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 30 m bgl **Screen interval 27 - 30 m bgl Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - overburden **Hydraulic conductivity (K) Well details Analysis details**

Bouwer-Rice 6.05 m/day

Initial water level 12.48 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 56 m bgl **Screen interval** 50 - 56 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - Interburden **Hydraulic conductivity (K) Well details Analysis details**

Well ID WMP22B **Test type** Slug test (Falling head test)

Early 0.02 m/day Bouwer-Rice

Late 0.01 m/day

Test type Slug test (Rising head test)

Well ID WMP22C **Test type** Slug test (Falling head test) **Initial water level** 12.98 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 206 m bgl **Screen interval** 200 - 206 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - underburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice 0.003 m/day

Well ID WMP22C **Test type** Slug test (Rising head test) **Initial water level** 12.98 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 206 m bgl **Screen interval** 200 - 206 m bgl **Inferred HSUs** Styx Coal Measures - underburden **Well details Analysis details Analysis details**

Time (sec)

Obs. Wells WMP22C Aquifer Model Unconfined Solution Bouwer-Rice **Parameters** K = 0.001851 m/day y0 = 0.3548 m

Well ID WMP23A **Test type** Slug test (Falling head test) **Initial water level** 13.25 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 56.5 m bgl **Screen interval** 48.5 - 54.5 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - interburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice 0.0007 m/day

Initial water level 4.80 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 26.4 m bgl **26.4** m bgl **H** α **H Screen interval** 23.4 - 26.4 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - overburden **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP24 **Test type** Slug test (Falling head test) 0.005 m/day Bouwer-Rice Hvorslev

0.006 m/day

Test data

Well depth 28 m bgl **Screen interval** 10.1 - 13.1 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP25 **Test type** Slug test (Falling head test) **Initial water level** 10.80 m btoc **Analysis solutions** Bouwer-Rice (1976)

Bouwer-Rice

0.006 m/day

Test data

Initial water level 15.40 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 20.5 m bgl **Screen interval** 11.5 - 20.5 m bgl **Inferred HSUs Alluvium**

Well details Analysis details

Well ID WMP26 **Test type** Slug test (Falling head test)

Well ID WMP27 **Test type** Slug test (Falling head test) **Initial water level** 20.99 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 21.7 m bgl **Screen interval** 14.5 - 20.5 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium / Styx Coal Measures - overburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice

0.006 m/day

Well ID WMP28 **Test type** Slug test (Falling head test) **Initial water level** 11.56 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 12 m bgl **Screen interval** 8.9 - 11.9 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Styx Coal Measures - overburden **Hydraulic conductivity (K)**

Well details Analysis details

Bouwer-Rice

0.002 m/day

Initial water level 12.78 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 12.5 m bgl **Screen interval** 6.5 - 12.5 m bgl **Inferred HSUs** Alluvium

Well details Analysis details

Well ID WMP29A **Test type** Slug test (Falling head test)

Initial water level 12.78 m btoc **Analysis solutions** Bouwer-Rice (1976) **Well depth** 12.5 m bgl **Screen interval** 6.5 - 12.5 m bgl **Analysis method** AQTESOLV **Inferred HSUs** Alluvium **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP29A **Test type** Slug test (Rising head test)

Bouwer-Rice

Early 7 m/day

Well depth 20 m bgl **Inferred HSUs** Alluvium **Hydraulic conductivity (K)**

Well details Analysis details

Well ID WMP29B **Test type** Slug test (Falling head test) **Initial water level** 13.03 m btoc **Analysis solutions** Bouwer-Rice (1976)

Screen interval 16 - 20 m bgl Analysis method AQTESOLV **Analysis method** AQTESOLV Early 0.2 m/day Mid 0.02 m/day Bouwer-Rice

Test data

Obs. Wells WMP29B Aquifer Model **Confined Solution** Bouwer-Rice **Parameters** $K = 0.1521$ m/day y0 = 0.3004 m

Well depth 20 m bgl **Screen interval** 16 - 20 m bgl **Inferred HSUs Alluvium**

Well details Analysis details Analysis details

Well ID WMP29B **Test type** Slug test (Rising head test) **Initial water level** 13.10 m btoc **Analysis solutions** Bouwer-Rice (1976)

Test data

